

More Possibilities. The Scaffolding System.

## LAYHER ALLROUND SCAFFOLDING<sup>®</sup> TECHNICAL BROCHURE



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## THE ORIGINAL ALLROUND SCAFFOLDING – AND ITS INGENIOUS ALLROUND TECHNOLOGY

The original Layher Allround Scaffolding using the principle of *rosette and wedge head* was registered for patent with the German Patent Office for the first time in 1974. An ingenious invention that revolutionised scaffolding technology in every respect!



**The original Layher Allround Scaffolding** offers particularly in structural and engineering scaffolding assembly – in applications with continually new and often unusual challenges – the right solutions. A persuasive variety of uses, rapid assembly and gratifying profitability at all times, not least thanks to an extensive range of series-produced expansion parts: this is the unrestricted versatility of Allround as a modular system.

The proven combination of positive and non-positive connections in rapid and bolt-free system technology with AutoLock function permits connections that are automatically right-angled, obtuse-angled and acute-angled as required, with built-in safety at the same time.

Layher Allround Scaffolding has become a synonym in the marketplace for modular scaffolding. With Layher Allround Scaffolding – in steel or in aluminium – you are investing in a perfected and complete system with all the approvals required for faster, safer, more profitable and highly flexible scaffolding construction.

# DIGITAL SCAFFOLDING PLANNING SIM | SCAFFOLDING INFORMATION MODELING

Digitalisation is affecting every industry. That includes scaffolding construction. Rightly so, because nothing else optimises project planning so effectively while opening up for you enormous potential for both transparency and cost savings. Layher therefore asked itself the question of how the BIM concept – Building Information Modeling – originating in civil engineering could be adapted to scaffolding in temporary structures. Because the proven Layher systems permit faster and safer upward access, yet are not part of the actual structure. Furthermore, scaffolding can also be used independently of civil engineering projects, for example as stand-alone structures like temporary bridges. The result is SIM: Scaffolding Information Modeling.

#### The future in scaffolding construction is digital - and it's name is SIM

Scaffolding Information Modeling – SIM for short – is a process based on 3D models and designed by Layher to meet the specific requirements of scaffolding construction. SIM not only allows you to plan, assemble and manage temporary scaffolding structures more efficiently, but also affords access to BIM at the same time. With the integrated Layher software solution "LayPLAN SUITE", you have a powerful tool for the SIM process: LayPLAN CLASSIC facilitates a start in digital planning by allowing automated planning of predefined scaffolding applications – and if required even with temporary roof structures. For complex scaffolding structures as part of large-scale engineering scaffolding, there is Lay-PLAN CAD. Detailed information on the modules of LayPLAN SUITE can be found on the following pages.

#### Planning and scheduling certainty at sites

Dependable 3D planning of scaffolding structures without collisions is just one of many benefits. Added to that are the realistic visualisation of scaffolding, allowing work to be coordinated with other trades or construction sequence simulation, transfer of the scaffolding planning to structural analysis programs, and output of material lists and assembly plans. Transparency at every work step results in a reduction in costs and an increase in safety and profitability. When they

### 1. LayPLAN CLASSIC for SpeedyScaf and Allround Scaffolding

LayPLAN CLASSIC facilitates a start in digital planning by allowing automated planning of predefined scaffolding applications: whether they're for circular or facade scaffolding made from SpeedyScaf, for birdcage scaffolding and free-standing towers made from Allround Scaffolding, or for structures with temporary roofs.

Once the key data has been entered, scaffolding manufacturers receive in seconds a scaffolding-proposal that includes anchoring, bracing and side protection. During the design phase, the overall length, standing heights and areas are continuously calculated and displayed to reflect the latest plan. A materials list can also be easily created at the push of a button: scaffolding erectors benefit from more certainty when planning the commercial and technical details; from optimised use of their stocks; and from full cost transparency at every stage of the project. work with Layher's scaffolding construction customers, both building contractors and end customers in industry benefit, with SIM, from a high degree of planning certainty, cost control and above all completion of projects on schedule thanks to efficient and undisrupted construction processes. Delays and added costs due to inadequate planning are a thing of the past.



### YOUR BENEFITS AT A GLANCE

- Transparency in all work steps and cost control.
- Increase in safety and in profitability for every project.
- Planning and scheduling certainty at every site.
- Your access to BIM.



3D visualisation in LayPLAN CLASSIC



Facade scaffolding with brick guard level and vehicle access using LayPLAN CLASSIC SpeedyScaf

### THE FUNCTIONS OF LAYPLAN CLASSIC

- Automated planning of standardised scaffolding structures using Speedy-Scaf, Allround Scaffolding and Layher weather protection roofs.
- Export function to LayPLAN CAD.



Planning of a weather protection roof with Keder Roof XL on Allround support scaffolding using LayPLAN CLASSIC

- > Automatic 2D drawings.
- > 3D visualisation for order acquisition.
- Real-time material list for transport and assembly.

#### 2. LayPLAN MATERIAL MANAGER for LayPLAN CLASSIC and LayPLAN CAD

The LayPLAN MATERIAL MANAGER allows material lists to be created and edited – for example splitting into different construction sections to permit prices and weights to be considered separately.



### 3. LayPLAN VR VIEWER

The free-of-charge LayPLAN VR Viewer enables virtual tours of scaffolding structures, to convey a realistic spatial impression of the overall situation. Based on the data from LayPLAN CAD, Layher can create for you VR models for display in the LayPLAN VR VIEWER. We'd be happy to assist you on the spot with our specialists and equipment for your VR presentation.

### THE FUNCTIONS OF LAYPLAN VR VIEWER

- > Virtual tours of scaffolding structures with VR headset (e.g. Oculus Rift).
- > Optional display of VR models in Desktop mode.
- Integrated measurement and comment function.
- Conveying of a realistic spatial impression of the overall situation, for order acquisition and for coordination with other trades or for construction sequence simulation.

#### THE FUNCTIONS OF THE LAYPLAN MATERIAL MANAGER

- Automatic creation of material lists from LayPLAN CLASSIC and LayPLAN CAD.
- Manual editing of material lists, for example splitting them into construction sections and applications.
- Detailed information on the scaffolding components including preview image.
- Output as PDF and export in Excel.
- Optional component images on the material lists in the printout this makes it easier to identify components during loading and assembly.



#### 4. LayPLAN CAD for planning in 3D

For complex scaffolding structures as part of large-scale engineering scaffolding, LayPLAN CAD is available. This is a plug-in for Autodesk AutoCAD. It permits 3-dimensional planning of scaffolding structures of all types.



Planning of individualised scaffolding structures in LayPLAN CAD



Creation of planning documents with integrated material lists in LayPLAN CAD



Professional 3D rendering of the LayPLAN CAD models

#### THE FUNCTIONS OF LAYPLAN CAD

- Scaffolding planning and design in 3D.
- Basic planning can be done in an automated process using the proven LayPLAN CLASSIC – that saves time.
- Dependable visual collision check thanks to realistic rendering as a volume model.
- Extensive component library with a convenient search function including prefabricated assemblies and template drawings for even faster design.
- Preview image of components and output as 3D models.
- Automatic component identifications.
- Real-time material list for transport and assembly the required material is guaranteed to be there where it's needed.
- Further editing of the model data in visualisation software (e.g. rendering, VR) for order acquisition and for coordination with other trades or for construction sequence simulation.
- Further editing of the model data in RSTAB for structural strength calculations as part of project-related verifications of stability. Unlike in remodeling which is otherwise necessary, this avoids error sources and saves time when planning. If you are interested, we'd be happy to send you supplementary Layher information for export into RSTAB.
- > Available in English, German, French and Spanish.

### 5. LayPLAN TO RSTAB

For structural strength verification of scaffolding structures, frame analysis programs are generally used. Using the LayPLAN TO RSTAB module, all modeling-relevant information about an Allround Scaffolding structure is imported three-dimensionally into the RSTAB frame analysis program from Dlubal. Automated transmission of the information means that re-entering the model data is not needed. This means that the user will benefit from an enormous time saving, and also avoid a possible source of errors during modeling.



Transmission of model data with the aid of LayPLAN TO RSTAB



Imported RSTAB model, prepared for structural strength computations

### THE FUNCTIONS OF LAYPLAN TO RSTAB

- Time saving thanks to automated 3D model transmission of Allround Scaffolding structures.
- Transmission of all structurally relevant information according to the German approvals (geometry, cross-sections, materials, frame types, eccentricities and non-linear connections).
- Avoidance of possible sources of errors during modeling in the frame analysis program.



Structural strength computations based on definition of nodal supports and loads

### PROCESSING OF THE MODEL DATA UP TO 3D USE IN SIM

Digital 3D scaffolding planning affords many advantages over planning in 2D as previously used: from a high degree of detail in planning and in drawings to the visual collision check and to professional visualisation of the scaffolding structure. The basis for scaffolding planning is 3D building model data. It is available as a rule from your customer as part of the BIM process. Alternatively, it

is possible to remodel the 3D building model data on the basis of 2D plans or manual building measurements or 3D scans – stationary or using a drone. Once 3D scaffolding planning with LayPLAN CAD is finished, the data can also be used without any problem for downstream processes, for example the creation of part lists or construction sequence simulation.



- Realistic 3D scaffolding planning.
- Visualisation of the design for professional presentation.
- Collision check.
- Data transfer to structural analysis programs.
- Material lists for logistic planning and costing.
- > 2D plans for assembly.
- Construction sequence simulation.
- VR model for virtual tour.
- Communication / data exchange with mobile devices.



Further information about SIM in the building trade can be found on film at: http://yt-sim-en.layher.com



Further information about SIM in industry can be found on film at: http://yt-sim-ind-en.layher.com



Further information about LayPLAN SUITE can be found on film at: **qrsoftwareen.layher.com** 

## THE ALLROUND POWER CONNECTOR MAKES IT

Whether it's used in industry, chemical plants, power stations, aircraft factories, shipyards, theatres or arenas, on every site and on every structure the "original" does justice to its reputation as an "Allrounder". As work scaffolding and safety scaffolding at the facade, as birdcage, trestle and suspended scaffolding, or as a rolling tower – the right scaffolding at all times and for every job and requirement. For very difficult ground plans and anchoring conditions, for very irregular structures, and for jobs with increased safety requirements.

## AUTOLOCK - FUNCTION IN THE ALLROUND LEDGER LW



**It's this easy:** Turning the ledger and slightly tilting it before assembly activates the AutoLock function.

As the wedge head is pushed over the rosette, the wedge drops automatically into the recess and the ledger end is **immediately secured against any possibility of shifting.** 

This means: safer 1-man assembly, whatever the height.



The flat rosette without recesses or bulges prevents it getting clogged with the dirt, of whatever type, that makes assembly difficult.



A hammer blow on the wedge transforms the positive connection into a non-positive one.

### THE ALLROUND POWER CONNECTOR





The wedge head is precisely matched to the radius of the standard at the front end – so forces are applied to a flat surface and always centrally into the standard.

Built-in assembly speed: the four narrow openings in the rosette automatically centre the ledgers in the correct dimensions and at right angles – the four wide openings permit alignment of ledgers and diagonal braces at the angles required.





## AN INGENIOUS DESIGN PRINCIPLE



Up to eight connections can be made in the structurally ideal Allround connector, on one level and at various angles. How the system is assembled is self-explanatory.



## **GUARANTEED WITH GERMAN APPROVAL**

SAFER. CERTIFIED. TESTED.

Z-8.22-939: THE ALLROUND LIGHTWEIGHT IN HIGH-TENSILE STEEL Z-8.22-64: THE ALLROUND MODULAR SYSTEM IN STEEL (VARIANT K 2000+ AND EARLIER VARIANTS [VARIANT I AND VARIANT II]) Z-8.22-949: THE MODULAR SYSTEM ALLROUND LWV IN STEEL (JOINT USE OF THE VERSIONS OF VARIANTS I TO LW)

The Layher Allround LW connector was developed by optimisation of the K2000+ variant and of the Allround connector registered for patent in 1974 and proven ever since. The Layher Allround LW connector offers, in comparison with the previous connector variant,

- substantially higher loading capacities
- e.g. bending moment of ledger connection:
   + 18.8 %

This means: even more possibilities.

Joint use with the Allround material of previous designs is generally assured and regulated by german approval.

That means: existing Allround material can remain in use without any restriction.





Layher Allround Scaffolding has, in addition to German approval, further approvals and certificates worldwide.

The Allround connector is the cutting edge: superb design, high-quality material and precision manufacture from Layher ensure high stability, dependable quality and greater safety.

Unless stated otherwise in the tables and lists, the load values (permissible loads, load classes, design resistances) quoted in this brochure are based on Layher's in-house calculations. They have been prepared to the best of our knowledge and belief by structural engineers qualified to do so.

The specifications of the following technical rulebooks provide the foundation for these calculations:

- DIN EN 12810-1:2004-03
- DIN EN 12811-1:2004-03 in conjunction with the "Application guideline for work scaffolding in accordance with DIN EN 12811-1"
- Eurocode 3: Design of steel structures
- Eurocode 9: Design of aluminium structures
  - (with exception of aluminium Allround Scaffolding components which were dimensioned on the basis of the "DIBt approval principles for the dimensioning of aluminium components in scaffolding construction", issued in May 1996)

along with the issues of the German Layher approvals applicable at the time of going to press.

## GERMAN APPROVALS FOR THE STANDARD ASSEMBLY

## SAFER. CERTIFIED. TESTED.

Z-8.22-939 / Z-8.22-64 / Z-8.22-949

## Approvals for the standard assembly as facade scaffolding.

The German approvals for Allround Scaffolding cover the connector and assembly as facade scaffolding. No vertical diagonal braces are required in the standard assembly for facade scaffolding according to the approval.

## At the facade too the Allround Scaffolding offers the proven Allround advantages:

- Low tendency to clogging
- "Automatic" right-angled assembly
- Flexibility
- High loading capacity
- Decks can be removed or installed at any point and at any time

#### Allround Scaffolding as an intelligent and economical solution.

Particularly irregular facades and structures with curving ground plan can be enclosed economically and safely using Allround Scaffolding.

This is where Allround Scaffolding with its persuasive adaptability offers an intelligent and economical solution.

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## ALLROUND CONNECTORS MADE OF ALUMINIUM SAFER. CERTIFIED. TESTED. Z-8.22-64.1

Possible applications in which the specific advantages of Layher Allround Scaffolding made of aluminium can be used to particular advantage in terms of both profitability and design include

- rolling towers
- suspended scaffolding
- as scenery in theatres
- in the trade fair and events field

#### And in addition,

- faster assembly
- less physical strain on the erectors
- Iow weight

are specific reasons for using the Layher Allround Scaffolding in aluminium.

#### Examples of typical applications:

- A setup area not sufficiently firm to sustain the weight of a steel scaffolding structure.
- Historic natural stone masonry starting to crumble due to environmental influences – has to be repaired and is no longer able to support steel scaffolding.
- In boilers, power stations etc. with manhole feeding where low weight is particularly important for ease of handling

All these are scenarios where use of Layher Allround Scaffolding of aluminium is appropriate.

## ALLROUND SCAFFOLDING – COMPONENTS

Three basic elements – standard, ledger, diagonal brace – in practically-minded dimensions, together with application-oriented expansion parts, make up the Allround system. All parts are made at Layher's own certified production facility, out of steel – hot-dip galvanized – or aluminium, depending on the function. Proven, high quality thanks to continuous monitoring, starting at goods reception and continuing during every phase of manufacture. Short delivery times from plentiful stocks, and reliable availability thanks to the special transporters in the company's own large vehicle fleet, as well as additional stocks held for you in a tight-knit network of delivery warehouses.

## VERTICAL SUPPORT ELEMENTS IN STEEL



Standard LW, steel with integrated spigot Length 0.50 m - 4.00 m Weight 2.7 kg - 18.1 kg Ref. No. 2617.xxx

**Standard LW**, steel without spigot Length 0.50 m – 3.00 m Weight 2.5 kg – 13.7 kg Ref. No. 2619.xxx

Allround ARGS standard LW Length 2.00 m Weight 8.0 kg Ref. No. 2602.065

**ARGS guardrail** Length 0.73 m – 3.07 m Weight 1.4 kg – 5.5 kg Ref. No. 2602.xxx









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HORIZONTAL SUPPORT ELEMENTS, SIDE PROTECTION



Allround O-ledger LW, steel with AutoLock function Length 0.25 m – 4.14 m Weight 1.4 kg – 13.4 kg Ref. No. 2601.xxx

**U-ledger LW T14**, steel Length 0.45 m - 1.40 m Weight 2.1 kg - 5.4 kg Ref. No. 2618.xxx

**U-ledger reinforced LW T14**, steel Length 1.40 m - 3.07 m Weight 8.9 kg, 19.0 kg Ref. No. 2618.xxx

U-ledger, steel deck/steel deck Length 0.32 m - 0.96 m Weight 3.1 kg - 5.5 kg Ref. No. 2614.xxx

 $\begin{array}{l} \textbf{U-ledger, steel deck/O-ledger} \\ \text{Length } 0.32\,\text{m} - 0.96\,\text{m} \\ \text{Weight } 3.3\,\text{kg} - 6.5\,\text{kg} \\ \text{Ref. No. 2614.xxx} \end{array}$ 



**U-ledger,** aluminium Length 0.73 m Weight 1.5 kg Ref. No. 3203.073

**U-ledger reinforced**, aluminium Length 1.09 m - 1.40 m Weight 3.7 kg, 4.5 kg Ref. No. 3203.109, 3203.140

**U-bridging ledger,** aluminium Length 1.57 m, 2.07 m Weight 4.3 kg, 5.5 kg Ref. No. 3207.157, 3207.207

**O-ledger reinforced LW**, steel Length 1.09 m – 3.07 m Weight 5.9 kg – 17.0 kg Ref. No. 2672.xxx

**U-lift-off preventer T8** Length 0.39 m - 1.29 m Weight 0.6 kg - 2.1 kg Ref. No. 2635.xxx

U-lift-off preventer T9

Length 1.40 m – 3.07 m Weight 5.3 kg – 11.9 kg Ref. No. 2658.xxx

#### U-toe board, wood

for decks with U-suspension, for longitudinal and end sides Length 0.73 m - 4.14 mWeight 1.5 kg - 7.5 kgRef. No. 2640.xxx

#### U-toe board, aluminium

for longitudinal and end sides, lightweight and durable Length 0.73 m - 3.07 m Weight 1.5 kg - 5.7 kg Ref. No. 2651.xxx

#### U-steel toe board

Length 0.73 m – 3.07 m Weight 1.8 kg – 6.3 kg Ref. No. 2644.xxx

#### Universal U-lift-off preventer

universally usable in any U-section (steel and aluminium), WS 19 and WS 22 Length 0.28 m Weight 1.0 kg Ref. No. 2635.xxx



### **DIAGONAL BRACING**

Diagonal brace LW, steel

For bay heights from 0.50 m - 2.00 mFor bay lengths from 0.73 m - 4.14 mWeight 3.9 kg-4.5 kg Ref. No. 2683.xxx, 2682.xxx, 2681.xxx, 2680.xxx

Diagonal brace, aluminium For bay lengths from 0.73 m - 3.07 mLength 2.12 m - 3.58 m Weight 3.9 kg - 5.3 kg Ref. No. 3204.xxx



## SCAFFOLDING DECKS, ACCESS DECKS

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U-steel deck LW, 0.32 m wide Length 0.73 m - 3.07 m Weight 5.6 kg - 19.3 kg Ref. No. 3883.xxx

U-steel deck T4, 0.32 m wide Length 0.73 m - 4.14 m Weight 6.0 kg - 29.8 kg Ref. No. 3812. xxx/3802.xxx



U-Xtra-N deck, 0.61 m wide Length 0.73 m - 3.07 m Weight 7.0 kg - 23.5 kg Ref. No. 3866.xxx

U-robust deck. 0.61 m wide Length 0.73 m - 3.07 m Weight 7.2 kg - 24.2 kg Ref. No. 3835.xxx

U-Xtra-N deck, 0.32 m wide Length 1.57 m – 3.07 m Weight 8.5 kg - 15.2 kg Ref. No. 3877.xxx

## U-Stalu deck T9, 0.61 m wide

Length 0.73 m - 3.07 m Weight 6.6 kg - 21.0 kg Ref. No. 3867.xxx



















U-Stalu deck T9, 0.32 m wide Length 1.57 m - 3.07 m Weight 7.4 kg - 13.3 kg Ref. No. 3856.xxx

U-Stalu deck T9, 0.19 m wide Length 1.57 m - 3.07 m Weight 5.6 kg - 10.2 kg Ref. No. 3857.xxx

U-Xtra-N access deck, 0.61 m wide, with integrated access ladder Length 2.57 m - 3.07 m Weight 25.4 kg - 29.5 kg Ref. No. 3869.xxx

U-Robust access deck, 0.61 m wide, with integrated access ladder Length 2.57 m - 3.07 m Weight 24.0 kg - 27.4 kg Ref. No. 3838.xxx

U-aluminium access deck, 0.61 m wide, with integrated access ladder Length 2.57 m - 3.07 m Weight 24.0 kg - 28.0 kg Ref. No. 3852.xxx

U-access deck, aluminium, 0.61 m wide, side-opening hatch, without ladder Length 2.07 m Weight 17.6 kg Ref. No. 3875.207

#### U-access steel deck, 0.64 m wide

access hatch of aluminium Length 2.07 m, 2.57 m Weight 28.9 kg, 38.0 kg Ref. No. 3813.207, 3813.257

U-access deck, aluminium, 0.61 m wide, without ladder Length 1.00 m Weight 10.0 kg Ref. No. 3851.100

#### Access ladder, 7-rung, T15/T19, steel

for access deck Length 2.15 m Weight 7.6 kg Ref. No. 4008.007/4009.007

Steel plank 0.20 m

hot-dip-galvanized Length 1.00 m - 2.50 m Weight 4.8 kg – 11.8 kg Ref. No. 3878.xxx

Steel plank 0.30 m hot-dip-galvanized Length 1.00 m - 2.50 m Weight 6.5 kg – 15.3 kg Ref. No. 3880.xxx

### Components







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## SCAFFOLDING SPINDLES



Locking screw, long (red), steel galvanized, for securing steel planks on steel decks, 50 pcs. Length 0.08 m, WS 19/22 Weight 4.0 kg, 3.9 kg Ref. No. 3800.009, 3800.010

Locking screw, short (blue), steel galvanized, for securing steel gap sheets on steel decks, 50 pcs. Length 0.04 m, WS 19/22 Weight 2.3 kg Ref. No. 3800.011, 3800.012

Steel gap sheet, 0.32 mUse up to load class 6 with a max. gap width of 20 cm For bay lengths from 0.73 m - 2.00 mWeight 2.6 kg - 12.0 kgRef. No. 3881.xxx

**U-gap sheet, with hooks,** 0.32 mFor bay lengths of 1.57 m - 3.07 mWeight 4.5 kg - 12.3 kgRef. No. 3882.xxx

#### Telescoping U-gap deck

closes gaps between 40 and 255 mm, infinitely adjustable Length 0.73 m - 3.07 m Weight 5.2 kg - 22.3 kg Ref. No. 3881.xxx

**U-gap ledger LW,** 0.11 m wide Length 0.73 m – 2.57 m Weight 5.2 kg – 17.6 kg Ref. No. 2675.xxx

Locking pin, plastic, dia. 11 mm once-only use, 100 pcs. Length 0.08 m Weight 0.1 kg Ref. No. 3800.006







### BRACKETS







Swiveling base plate 60, reinforced (max. spindle travel 32 cm), ensure sufficient structural strength Length 0.58 m, Weight 6.1 kg Ref. No. 4003.000

**Head jack 45**, solid, 16 cm (max. spindle travel 26 cm) Fork width 16 cm Length 0.45 m Weight 6.6 kg Ref. No. 5314.045

## Wedge spindle swivel coupler

Weight 1.8 kg Ref. No. 4735.000

**Base collar** Length 0.24 m Weight 1.4 kg Ref. No. 2602.000

#### U-bracket LW, 0.28 m wide

for U-deck 0.19 m wide U-lift-off preventer provided by customer Length 0.28 m Weight 3.4 kg Ref. No. 2632.019

U-bracket LW, 0.39 m wide

for U-deck 0.32 m wide Length 0.39 m Weight 3.9 kg Ref. No. 2632.039

U-bracket LW, 0.73 m wide for 2 U-decks 0.32 m or 1 U-deck 0.61 m wide Length 0.73 m Weight 6.4 kg Ref. No. 2632.073

Bracket brace Length 2.05 m

Weight 8.8 kg Ref. No. 2631.205

LATTICE BEAMS





#### **O-lattice beam LW**, with 4 wedge heads, steel Length 2.07 m - 7.71 m Weight 22.2 kg - 71.0 kg Ref. No. 2674.xxx

U-lattice beam LW, with 4 wedge heads, steel Length 2.07 m - 6.14 m Weight 21.4 kg - 60.5 kg Ref. No. 2673.xxx

### **Base plate 60** (max. spindle travel 41 cm) Length 0.56 m

Weight 3.6 kg Ref. No. 4001.060

Base plate 80, reinforced (max. spindle travel 55 cm) Length 0.73 m Weight 4.9 kg Ref. No. 4002.080





#### **U-lattice beam**, with 4 wedge heads Aluminium Length 1.57 m – 5.14 m Weight 8.6 kg – 30.2 kg Ref. No. 3206.xxx

Spigot for U-section / Spigot for U-section reinforced for lattice beam, incl. 2 bolts also for U-bridging ledger Weight 2.1 kg Ref. No. 2656.001/2656.002

Spigot for O-lattice beam with half-coupler, for lattice beam and ledger Weight 1.8 kg, WS 19/22 Ref. No. 4706.019, 4706.022

## STAIR ACCESS







**U-platform stair,** 2.00 m high, 0.64 m wide / 0.94 m wide Aluminium

Stair class A as per EN 12811-1 for 2.57 m and 3.07 m bay lengths Weight 21.9 kg – 40.1 kg Ref. No. 1753.xxx

**U-platform stair,** 1.50 m high 0.64 m wide / 0.94 m wide Aluminium Stair class A as per EN 12811-1 for 2.57 m bay length Weight 21.5 kg, 36.6 kg Ref. No. 1753.251, 1753.252

U-comfort stair, 2.00 m high 0.64 m wide Aluminium Stair class B as per EN 12811-1 for 2.57 m and 3.07 m bay length Weight 27.0 kg, 32.0 kg Ref. No. 1755.257, 1755.307

**Stair guardrail,** 2.00 m high with swiveling wedge heads for 2.57 m and 3.07 m bay length Weight 18.1 kg, 20.1 kg Ref. No. 2638.258, 2638.308



## COUPLERS







### ANCHORING



Stair guardrail, 2.00 m high with U-forks, steel galvanized for 2.57 m and 3.07 m bay length Weight 18.1 kg, 20.1 kg Ref. No. 2638.257, 2638.307

### Stair guardrail adapter

Weight 0.7 kg Ref. No. 2637.000

Wedge head coupler, rigid WS 19/22 Weight 1.1 kg, 1.1 kg Ref. No. 2628.019, 2628.022

Wedge head coupler, swiveling WS 19/22 Weight 1.5 kg, 1.5 kg Ref. No. 2629.019, 2629.022

Wedge head coupler LW, double Weight 1.2 kg Ref. No. 2629.000

**Rosette, clampable** WS 19/22 Length 0.12 m, 0.12 m Weight 1.1 kg, 1.2 kg Ref. No. 2602.019, 2602.022

#### Rosette with thread, clampable

WS 19/22 Length 0.12 m, 0.12 m Weight 1.7 kg, 1.7 kg Ref. No. 2602.119, 2602.122

Allround wall tie, 0.80 m Length 0.80 m Weight 3.3 kg Ref. No. 2639.080

## THE ASSEMBLY

The Allround wedge head system provides positive connection to every joint between standards, ledgers and diagonal braces as soon as they are assembled. This fundamental security stays with the assemblers and users of the scaffolding all the way up. The required non-positive connection is achieved with the specified hammer blow using a hammer of at least 500 g until the blow bounces.

6a

### LAYING OUT THE SCAFFOLDING



 Position spindles in the configuration dimension. Use load-distributing bases if the ground is not sufficiently firm.

Permissible loads and maximum spindle extension lengths must be complied with (see loading tables for base plates).

- 2 Push the base collar onto the base plate.
- **3** Connect the base collars in the longitudinal and transverse directions by ledgers in the selected configuration dimension.

For **right-angled** connections use the **small holes** of the rosette.

Then align the scaffolding base level horizontally, starting at the highest ground point, by adjusting the spindle nuts.

- 4 Attach standards and connect them at a maximum height of 2.0 m using a transversal ledger and scaffolding decks. In scaffolding levels without decks, longitudinal ledgers must be installed. Depending on static requirements, for example in some facade scaffolding assembly variants, install a further transversal ledger 0.5 m above the bottom transom.
- 5 We recommend in common applications that the standard lengths be selected such that the joints are on one deck level or on one braced ledger level. Diverging arrangements of the joints must be structurally verified. Note: When the ARGS is used, there is a different arrangement of standard joints. For facade scaffolding in the regular version, no longitudinal ledgers are required in deck levels with scaffolding decks.



6 Install diagonal braces according to the static requirements. Diagonal braces are not required in the standard assembly according to the German approval. If diagonal braces are required, they can be installed in tower form 6 a or continuously 6 b.



Diagonal braces continuous

Diagonal braces unidirectional in tower form.

The illustrations show the usual diagonal bracing arrangement: one diagonal brace for every 5 scaffolding bays; anchoring not shown.





7 All wedge connections must be knocked in with a hammer of at least 500 g until the blow bounces.

> If no scaffolding decks are installed, longitudinal ledgers must be installed, and in every 5th bay, ledgers as horizontal-diagonal bracing. This also applies for plank decking.

8 Assembly is continued by repeating the steps 4, 5, 6 and 7.

Insert scaffolding decks as bracing every 2.00 m apart in the upward direction as building work progresses.







## THE SCAFFOLDING DECK

In the Layher system, choose from decks made of steel, aluminium or an aluminium frame with glass-fibre-reinforced plastic or plywood board depending on the type of application and load class, but also in accordance with your working requirements. Common to all Layher decks is their horizontally bracing effect inside the scaffolding.



#### U-scaffolding decks

9/10 Suspend decks in U-ledger and secure them with U-lift-off preventer. Select the deck depending on load and bay width.





#### O-scaffolding decks

11 Place decks onto the O-ledger with the lift-off preventer swung back. Swing the lift-off preventer forwards.

Select the scaffolding decks depending on load and bay width.





12b Whenever the Allround Guardrail System (ARGS) is used, Allround ARGS standards LW
2.0 m 12 c and swiveling ARGS guardrails 12 d are used too. The standard joint of the ARGS standards is at 1.0 m height between the scaffolding levels. The ARGS system permits safer and faster assembly of the guardrails from the secured level underneath.



**13/14** Insert longitudinal and end toe boards behind the wedges.

### THE 3-PART SIDE PROTECTION.



**12a** Install ledgers 0.5 m above the deck level as an intermediate rail and 1.0 m above the deck level as a guardrail.

Install toe boards on the longitudinal side and at the ends of the deck level.



## THE PRACTICAL CORNER DESIGN



15 b install at every deck level ledger reinforced, LW or bridging ledger as shown. Lay decks and secure them with appropriate lift-off pre-

## CANTILEVERS





### Bracket cantilever

- 18 0.3 m bracket cantilever using Allround bracket and scaffolding decks
- 19 0.7 m bracket cantilever using Allround bracket, bracket brace and scaffolding decks.

#### Allround scaffolding cantilever

**20** Support cantilevers 0.5 m below the deck level with bracket braces or Allround diagonal braces. Safeguard scaffolding decks on cantilever structures from being inadvertently lifted out using appropriate lift-off preventer.

### TOWER AND BIRDCAGE SCAFFOLDING





16/17 Use of ledgers reinforced, LW, bridging ledgers and lattice beams.

## **ALLROUND BRIDGING**

Bridging of spans of up to 4.14 m can be achieved with steel decks of 4.14 m length together with 4.14 m long guardrails and toe boards. Bridging of gate entrances, building projections, balconies or openings using Allround lattice beams (see bridging variant A) or with vertical diagonal braces (see bridging variant B).



#### 21 Allround lattice beam:

Connect the wedge heads of the lattice beams to the rosettes of the vertical standards. For bridging variant A.

## BRIDGING ARRANGEMENTS FOR FACADE SCAFFOLDING

#### Bridging variant A

for load class 3, scaffolding width 0.73 m up to 24 m high



- Anchoring point for bridging
- T Scaffolding tube dia. 48.3 as per EN 39
- ① Ledgers inside and outside

### Bridging variant B1

for load class 3, scaffolding width 0.73 m up to 24 m high



#### Bridging variant B2

for load class 4, scaffolding width 1.09 m up to 24 m high, with diagonal braces K 2000+ or diagonal braces LW



• Anchoring point for bridging

- T Scaffolding tube dia. 48.3 as per EN 39 as horizontal diagonal brace
- ① Ledgers inside and outside

Position of the vertical diagonal braces:

\_\_\_\_\_ outside \_ \_ \_ \_ \_ inside

## DESIGN RESISTANCES PER GERMAN APPROVAL DESIGN RESISTANCES IN ALLROUND LEDGER CONNECTION

DESIGN RESISTANCES OF DIAGONAL BRACES TO NORMAL FORCE

## Z-8.22-939: LIGHTWEIGHT

#### **Bending moment**



#### Horizontal shear force



U-ledger:  $V_{y, Rd} = \pm 16.6 \text{ kN}$ 

Bending moment

 $M_{y, Rd} = \pm 120.0 \text{ kNcm}$ 





Vertical shear force single connection  $V_{z, Rd} = \pm 31.7 \text{ kN}$ Vertical shear force per rosette  $\sum V_{z, Rd} = \pm 117.0 \text{ kN}$ 





Design resistances of vertical diagonal braces LW for bay height 2.0 m									
Bay length [m]	0.73	1.036	1.09	1.40	1.57	2.07	2.57	3.07	4.14
Compression N <sub>V,Rd</sub> [kN]	-18.6	-19.9	-20.1	-18.6	-17.6	-14.4	-11.7	-9.5	-6.0
Tension $N_{v,Rd}[kN]$	+20.9	+24.2	+24.7	+25.6	+26.3	+28.5	+30.9	+32.2	+29.7

Design resistances of vertical diagonal braces LW for bay height 1.5 m										
Bay length [m]	0.73	1.09	1.57	2.07	2.57	3.07				
Compression N <sub>V,Rd</sub> [kN]	-19.4	-21.3	-22.5	-17.8	-13.9	-10.8				
Tension $N_{v,Rd}[kN]$	+23.0	+25.6	+28.3	+31.6	+31.3	+29.9				

Design resistances of vertical diagonal braces LW for bay height 1.0 m										
Bay length [m]	0.73	1.09	1.57	2.07	2.57	3.07				
Compression N <sub>V,Rd</sub> [kN]	-21.0	-23.2	-18.7	-17.1	-15.9	-12.1				
Tension $N_{v,Rd}[kN]$	+25.3	+28.2	+32.2	+30.0	+28.7	+28.1				

Design resistances of vertical diagonal braces LW for bay height 0.5 m										
Bay length [m]	0.73	1.09	1.57	2.07	2.57	3.07				
Compression N <sub>V,Rd</sub> [kN]	-21.1	-17.2	-16.1	-15.7	-15.5	-13.0				
Tension $N_{v,Rd}[kN]$	+30.4	+30.1	+28.2	+27.4	+27.1	+26.9				



V



V.

 $N_{Rd} = \pm 35.1 \text{ kN}$  for connection in large hole

#### **Torsional moment**



 $M_{T, Rd} = \pm 52.5 \text{ kNcm}$ 



## Z-8.22-64: K 2000+

#### Bending moment



Bending moment  $M_{y, Rd} = \pm 101.0 \text{ kNcm}$ 

Horizontal shear force



 $0\text{-ledger}\;V_{\text{y, Rd}} = \pm\;10.0\;\text{kN}$ 



U-ledger:  $V_{y, Rd} = \pm 5.9 \text{ kN}$ 

Normal force



Connection of O- and U-ledgers:  $N_{\textrm{Rd}}=\pm$  31.0 kN for connection in large and small hole

**Torsional moment** 



Vertical shear force



 $\begin{array}{l} \mbox{Vertical shear force} \\ \mbox{single connection} \\ \mbox{V}_{z,\,\mbox{Rd}} = \pm \ 26.4 \ \mbox{kN} \\ \mbox{Vertical shear force per} \\ \mbox{vertical shear force per} \\ \mbox{rosette} \\ \mbox{} \sum \ \mbox{V}_{z,\,\mbox{Rd}} = \pm \ 105.6 \ \mbox{kN} \end{array}$ 

Normal force, diagonal brace



Design resistances of vertical diagonal braces K2000+ for bay height 2.0 m										
Bay length [m]	0.73	1.036	1.09	1.40	1.57	2.07	2.57	3.07	4.14	
Compression $N_{V,Rd}[kN]$	-16.6	-17.9	-17.7	-16.3	-15.4	-12.8	-10.5	-8.5	-5.4	
Tension $N_{V,Rd}[kN]$	+18.0	+20.8	+21.2	+22.0	+22.6	+24.5	+26.7	+27.6	+25.5	

Design resistances of vertical diagonal braces K2000+ for bay height 1.5 m										
Bay length [m]	0.73	1.09	1.57	2.07	2.57	3.07				
Compression $N_{V,Rd}[kN]$	-17.8	-20.4	-19.3	-15.5	-12.3	-9.7				
Tension $N_{V,Rd}[kN]$	+19.8	+22.0	+24.4	+27.3	+26.8	+25.6				

Design resistances of vertical diagonal braces K2000+ for bay height 1.0 m										
Bay length [m]	0.73	1.09	1.57	2.07	2.57	3.07				
Compression N <sub>v,Rd</sub> [kN]	-20.0	-23.1	-18.7	-17.1	-14.0	-10.8				
Tension $N_{v,Rd}[kN]$	+21.7	+24.3	+27.6	+25.7	+24.6	+24.1				

Design resistances of vertical diagonal braces K2000+ for bay height 0.5 m										
Bay length [m]	0.73	1.09	1.57	2.07	2.57	3.07				
Compression N <sub>V,Rd</sub> [kN]	-21.1	-17.2	-16.1	-15.7	-15.2	-11.5				
Tension $N_{v,Rd}[kN]$	+26.2	+25.8	+24.1	+23.5	+23.2	+23.1				

### Z-8.22-64: Variant II

(Vertical standards, ledgers and diagonal braces of earlier design)

#### **Bending moment**



Bending moment  $M_{y, Rd} = \pm 68.0 \text{ kNcm}$ 

Vertical shear force



Normal force, diagonal brace

Vertical shear force single connection  $V_{z,\,Rd}=\pm~17.4~kN$ Vertical shear force per rosette  $\sum V_{z, Rd} = \pm 69.5 \text{ kN}$ 

#### Horizontal shear force



 $\text{O-ledger V}_{\text{y, Rd}} = \pm \ 6.7 \ \text{kN}$ 



Design resistances of vertical diagonal braces, Variant II for bay height 2.0 m								
Bay length [m]	0.73	1.09	1.40	1.57	2.07	2.57	3.07	4.14
Compression $N_{v,Rd}[kN]$		-8.4						
Tension N <sub>V.Bd</sub> [kN]				+8	.4			



Normal force



Connection of O- and U-ledgers:  $N_{\text{Rd}} = \pm$  22.7 kN for connection in large and small hole

"Permissible loads" or "safe working loads" are obtained by dividing the design resistance by 1.5 (=  $\gamma_{\rm F}$ ). The index Rd stands for design resistance.

## JOINT USE COMPONENTS OF DIFFERENT ALLROUND SCAFFOLDING GENERATIONS

The components of different generations of Allround Scaffolding may be used together without restriction. This is regulated in the German general building authority approvals Z-8.22-64 and Z-8.22-949.

In accordance with these approvals, the following regulations apply for the structural analysis of scaffolding structures containing components from different Allround Scaffolding generations:

Combination	Design re	sistances	Stiffnesses			
Allround Scaffolding components	Ledger connections	Vertical diagonal braces	Ledger connections <sup>3)</sup>	Vertical diagonal braces		
Variant II + K2000+	as Variant II	as Variant II <sup>1)</sup>	as K2000+			
LW + Variant II + K2000+ and LW + Variant II	as Variant II	as Variant II <sup>2)</sup>	as Variant II	as (Variant II and K2000+) 4)		
LW + K2000+	as K2	000+	as K2000+			

<sup>1)</sup> If vertical diagonal braces K2000+ are used with standards of Variant II, the values approved for them in accordance with Z-8.22-64 may be used alternatively.

<sup>21</sup> If only vertical diagonal braces LW and / or K2000+ are used, the values approved for them may be used alternatively, see <sup>1</sup> and Z-8.22-949

<sup>3)</sup> The ledger connections may – as in all Allround Scaffolding structures – also assumed to be articulated.

<sup>4)</sup> Note: Vertical diagonal braces Variant II and vertical diagonal braces K2000+ have the same stiffnesses.

The use of Allround Scaffolding components of the first generation, Variant I, is also permissible together with Allround Scaffolding components of Variant II, K2000+ and LW without restriction. Regulations regarding the stiffnesses and design resistances of the ledger connections and diagonal braces can be found in the above approvals.

## LOADING TABLES FOR ALLROUND STEEL

### ALL SPECIFIED LOADS ARE SAFE WORKING LOADS.

O-ledger	LW
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Permissible normal force O-ledger LW							
Ledger length [m]	Compressi	on N <sup>(.)</sup> [kN]	Tension N <sup>(+)</sup> [kN]				
	Connection in small hole	Connection in large hole	Connection in small hole	Connection in large hole			
≤ 1.57	-28.2	-23.4					
2.07	-27.3	-23.4	. 20 2	. 22 /			
2.57	-18	8.1	+20.2	+23.4			
3.07	-12	2.9					

0-ledger K2000+				ç
	<u>N</u> <sup>(-)</sup> В	N <sup>(-)</sup>	N <sup>(+)</sup>	N (+)

Permissible normal force O-ledger K2000+						
Ledger length [m]	Compression N <sup>(-)</sup> [kN]	Tension N <sup>(+)</sup> [kN]				
≤ 2.07	-20.7					
2.57	-19.1	+20.7				
3.07	-13.8					





Permissible normal force O-ledger Variant II						
Ledger length [m]	Compression N <sup>(-)</sup> [kN]	Tension N <sup>(+)</sup> [kN]				
≤ 2.57	-15.1	. 1E 1				
3.07	-13.8	+10.1				

O-ledger LV on LW stan	<b>W</b> dards		Š					
	Permissible load of O-led	lger LV	V					
	Ledger length (system dimension) [m]	0.73	1.09	1.40	1.57	2.07	2.57	3.07
<u>e</u>	Uniformly distributed line load (q) [kN/m]	29.2	14.1	8.8	7.0	4.1	2.7	1.9
	Concentrated load (P) in bay centre [kN]	10.1	7.1	5.7	5.1	4.0	3.3	2.7

### 0-ledger K 2000+

on K 2000+ standards

	Permissible load of 0-ledger K2000+								
	Ledger length (system dimension) [m]	0.73	1.09	1.40	1.57	2.07	2.57	3.07	
<b>q</b>	Uniformly distributed line load (q) [kN/m]	22.1	10.4	6.5	5.3	3.1	2.1	1.5	
	Concentrated load (P) in bay centre [kN]	7.4	5.2	4.2	3.8	3.0	2.4	2.1	

#### U- and O-bridging ledgers

on standards LW, K2000+ and Variant II

Permissible load of U- /O-bridging ledgers										
Ledger type [m]	U 1.57	U 2.07	U 2.57	U 3.07	0 1.57	0 2.07	0 2.57	0 3.07		
Uniformly distributed line load (q) [kN/m]	15.2	8.7	5.1	3.6	14.5	8.6	5.4	3.6		
Concentrated load (P) in bay centre [kN]	8.0	6.9	5.3	5.2	10.6	6.9	4.6	3.6		

0- and U-bridging ledgers are available in Variant K2000+ and Variant II

#### **O-ledger LW**

on K 2000+ standards

	Permissible load of O-ledger LW								
	Ledger length (system dimension) [m]	0.73	1.09	1.40	1.57	2.07	2.57	3.07	
<u>q</u>	Uniformly distributed line load (q) [kN/m]	29.2	14.1	8.8	7.0	4.1	2.3	1.5	
	Concentrated load (P) in bay centre [kN]	10.1	7.1	5.7	5.1	4.0	3.3	2.7	

### **O-ledger Variant II**

on standards Variant II

	Permissible load of O-ledger Variant II								
a	Ledger length (system dimension) [m]	0.73	1.09	1.40	1.57	2.07	2.57	3.07	
	Uniformly distributed line load (q) [kN/m]	22.1	8.8	4.6	3.5	1.8	1.1	0.7	
<sup>₽</sup>	Concentrated load (P) in bay centre [kN]	7.4	5.2	4.1	3.5	2.4	1.8	1.4	

<u>8</u>\_

#### Diagonal braces, H = 2,0 m

Bay length [m]	0.73	1.09	1.40	1.57	2.07	2.57	3.07
Compression force [kN]	-12.4	-13.4	-12.4	-11.7	-9.6	-7.8	-6.3
							0.4
Tension force [kN] Permissible load of v	+13.9 ertical	+16.5 diagoi	+17.1 nal bra	+17.5 Ices K	+19.0 2000+	+20.6 , H=2	+21. 2.0 m
Tension force [kN] Permissible load of v Poy leagth [m]	+13.9 ertical	+16.5 diago	+17.1 nal bra	+17.5	+19.0 2000+	+20.6 , H=2	+21.8 2.0 m
Tension force [kN] Permissible load of v Bay length [m] Compression force	+13.9 ertical 0.73 -11.1	+16.5 diagou 1.09 -11.8	+17.1 nal bra 1.40 -10.9	+17.5 Ices K 1.57 -10.3	+19.0 2000+ 2.07 -8.5	+20.6 , <b>H=2</b> 2.57 -7.0	+21. 2.0 m 3.07 -5.7

Permissible load of v	ertical	l diago	onal bi	races	Varian	t <b>II</b> = 2	2.0 m
Bay length [m]	0.73	1.09	1.40	1.57	2.07	2.57	3.07
Fension / compres- sion force [kN]	±5.6	±5.6	±5.6	±5.6	±5.6	±5.6	±5.6

#### U-ledger/U-ledger reinforced/O-ledger reinforced/U-ledger LW on standards LW, K2000+ and Variant II



	Permissible load of U-led	lger (U	), reinf	orced	ledger	(V), O-I	ledger	(0)
	Ledger type	U	U-V	U-V	0-V	0-V	U-LW	U-LW
	Length [m]	0.73	1.09	1.40	1.09	1.29	1.09	1.40
<u> </u>	Uniformly distributed line load (q) [kN/m]	19.0	17.3	10.4	21.8	15.6	17.5	10.8
~	Concentrated load (P) in bay centre [kN]	6.1	8.8	6.8	11.0	9.3	8.6	6.4

U-ledger reinforced LW/ O-ledger reinforced LW on standards LW and K2000+  $\,$ 



	Permissible load of U-	/ O-ledą	jer LW	reinfor	ced							
	Ledger type			U-LW-V					0-L'	W-V		
	Length [m]	1.40	1.57	2.07	2.57	3.07	1.09	1.40	1.57	2.07	2.57	3.07
<u>q</u>	Uniformly distributed line load (q) [kN/m]	19.8	17.7	13.0	8.4	5.0	21.4	17.1	16.1	11.1	8.5	6.0
<b>,</b>	Concentrated load (P) in bay centre [kN]	19.2	17.1	12.9	10.4	8.7	19.6	19.4	17.3	13.2	10.7	9.0

## ALLROUND O-LATTICE BEAM LW

	Permissible load of Allround O-lattice beam LW														
	Beam length [m]	2.07	2.57	3.07	4.14	5.14	6.14	7.71							
	Bracing of top chord	А	В	С	D	E	F	G							
q	Uniformly distributed line	21.6 <sup>A1</sup>	11.3 <sup>B1</sup>	5.5 <sup>C1</sup>	9.5	3.6 <sup>E1</sup>	3.4 F1	1.3 <sup>G1</sup>							
	load (q) [kN/m]	21.6 A2	17.7 <sup>B2</sup>	14.1 <sup>C2</sup>	0.0	7.7 <sup>E2</sup>	6.2 F2	4.5 <sup>G2</sup>							
P	Concentrated load (P) in bay centre [kN]	26.9 <sup>A1</sup>	14.2 <sup>B1</sup>	8.3 <sup>C1</sup>	25.9	13.6 E1	10.3 F1	5.1 <sup>G1</sup>							
<u> </u>		35.3 <sup>A2</sup>	37.2 <sup>B2</sup>	$[13.9^1/32.4^2]$ <sup>C2</sup>	20.0	27.3 E2	21.7 F2	17.1 <sup>G2</sup>							
P1 P1	Two concentrated loads (P1)	_						3.9 <sup>G1</sup>							
	in the one-third points [kN]		_	_	_	_	_	12.8 G2							

## ALLROUND O-LATTICE BEAMS K2000+ AND VARIANT II

	Permissible load of Allround	d O-lattice beams K	2000+ and Variant	II				
	Beam length [m]	2.07	2.57	3.07	4.14	5.14	6.14	7.71
	Bracing of top chord	A1	В	С	D	E	F	G
_q	Uniformly distributed line	16.7	11.0 <sup>B1</sup>	5.5 <sup>C1</sup>	7.2	3.6 E1	3.4 F1	1.3 <sup>G1</sup>
	load (q) [kN/m]	10.7	12.7 <sup>B2</sup>	10.1 <sup>C2</sup>	7.5	5.5 <sup>E2</sup>	4.5 F2	3.3 G2
P	Concentrated load (P) in	25.4	14.2 <sup>B1</sup>	8.3 <sup>C1</sup>	25.0	13.6 E1	10.3 F1	5.1 <sup>G1</sup>
<u> </u>	bay centre [kN]	20.4	26.7 <sup>B2</sup>	$[11.2^{1}/23.3^{2}]^{C2}$	23.0	23.4 E2	18.8 F2	14.8 <sup>G2</sup>
P <sub>1</sub> P <sub>1</sub>	Two concentrated loads (P1)							3.9 <sup>G1</sup>
<u>↓</u> ↓	in the one-third points [kN]	_	_	_	_	_	_	11.1 <sup>G2</sup>

 $^1$  Concentrated load exactly in the centre of the lattice beam (= between the two central posts)  $^2$  Concentrated load above one of the central posts

## BRACING OF THE LATTICE BEAMS WITH TUBES AND COUPLERS



ALLROUND U-LATTICE BEAM LW, K2000+ Permissible loads when the lattice beams are completely covered with U-decks secured with lift-off preventer

	Permissible load of Allro	und U-lattice beams LV	V and K2000+				
	Beam length [m]	2.07	2.57	3.07	4.14	5.14	6.14
<u>q</u>	Uniformly distributed line load (q) [kN/m]	17.3	12.5	10.2	7.3	5.2	4.3
,	Concentrated load (P) in bay centre [kN]	25.1	26.6	8.2 <sup>1</sup> /19.5 <sup>2</sup>	16.2	15.9	10.9

Permissible loads when the lattice beams are braced with a combination of tubes and couplers or when the lattice beams are not braced

	Permissible load of Allro	und U-lattice beams LV	V and K2000+				
	Beam length [m]	2.07	2.57	3.07	4.14	5.14	6.14
	Bracing of top chord	А	В	С	D	E	F
	Uniformly distributed line load (q) [kN/m]	17.3	12.5	7.5 <sup>C1</sup> 10.2 <sup>C2</sup>	7.3	4.6 <sup>E1</sup> 5.2 <sup>E2</sup>	2.4 F1 4.3 F2
<u>⊾</u>	Concentrated load (P) in bay centre [kN]	25.1	17.9	$\frac{(8.2^{1}/11.3^{2})^{\text{C1}}}{(8.2^{1}/19.5^{2})^{\text{C2}}}$	16.2	15.9 <sup>E1, E2</sup>	10.9 <sup>F1, F2</sup>

 $^1$  Concentrated load exactly in the centre of the lattice beam (= between the two central posts)  $^2$  Concentrated load above one of the central posts

## BRACING OF THE LATTICE BEAMS WITH TUBES AND COUPLERS

Lattice beam 2.07 m	Lattice beam 2.57 m	Lattice beam 3.07 m	Lattice beam 4.14 m
A: No bracing	B: No bracing	C1: No bracing	D: In the centre, at the post*
Lattice beam 5.14 m E1: In the centre, at the post*	Lattice beam 6.14 m F1: In the centre, at the post*	<ul> <li>Bracing at the posts means: longitudinal tubes at the posts, connected directly underneath the top chord. The horizontally / diagonally running tubes are connected to the longitudinal tubes.</li> <li>Horizontally / diagonally running tubes in at least every 5th bay.</li> </ul>	

E2 : At all posts\*

2

F2: At all posts\*





Support scaffolding including its bracing and side protection is not illustrated.

### BASE PLATE 60 LOADING TABLE



Equivalent section properties of the thread  $A = 3.84 \ cm^2 \\ W_{el} = 2.61 \ cm^3 \\ W_{pl} = 3.26 \ cm^3$ 

 $I = 3.74 \text{ cm}^4$ 

Material: EN 10219-S235JRH

→ Rolled thread:  $f_{y,k} = 280.0 \text{ N/mm}^2$ 

ermissible load of base plate 60																						
			in ca	se of	a siı	nulta	ineoi	Perr usly a	nissi Ictin	ble v g hor	ertica izont	al loa al loa	dN adH	[kN] [kN]	for d	liffer	ent li	ft hei	ights			Perm. hori- zontal load
Extension I <sub>a</sub>	ŀ	l = 0.	0	ŀ	=1.	0	ŀ	l = 2.0	)	ŀ	<b>I</b> = 3.	0	ŀ	<b>I</b> = 4.0	D	ŀ	l=5.	)	ŀ	<b>I = 6</b> .	0	H [kN],
[]										Le	vel [r	n]										when
	2.0	1.5	1.0	2.0	1.5	1.0	2.0	1.5	1.0	2.0	1.5	1.0	2.0	1.5	1.0	2.0	1.5	1.0	2.0	1.5	1.0	N = U KN
0	39 <sup>1</sup>	53	59	39 <sup>1</sup>	51	58	39 <sup>1</sup>	50	57	39 <sup>1</sup>	49	55	38	47	54	36	45	52	35	43	51	26.3
5	39 <sup>1</sup>	52	58	39 <sup>1</sup>	50	56	38	47	54	36	44	51	34	42	49	32	39	46	30	37	42	7.8
10	39 <sup>1</sup>	51	57	38	47	52	36	43	46	33	40	40	29	36	33	26	31	25	24	23	16	4.6
15	39 <sup>1</sup>	49	54	36	44	46	33	39	37	29	30	27	24	20	16	-	-	-	_	-	-	3.2
20	38	47	49	34	40	39	29	29	27	-	17	15	-	-	-	-	-	-	-	-	-	2.5
25	37	44	44	31	33	32	22	20	19	-	-	-	-	-	-	-	-	-	-	-	-	2.0
30	35	38	39	27	26	26	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	1.7
35	32	33	34	21	20	20	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	1.5
37	30	31	31	17	18	18	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	1.4

The permissible vertical loads are calculated by application of the calculation model according to DIN EN 12811-1, para. 10.2.3.2. To consider the bending stiffness of the Allround standard, the effects from second-order theory and the maximum load-bearing capacity of the standards, birdcage scaffolding with modular dimension 2.57 x 2.57 m and different lift heights was considered.

(-) With this combination of spindle extension and horizontal load, the load-bearing capacity of the spindle is exceeded.

<sup>1</sup> Here the permissible vertical load of the standard at 2.0 m lift height is reached (39 kN)









### BASE PLATE 80 REINFORCED LOADING TABLE

E



Equivalent section properties of the thread A = 4.71 cm<sup>2</sup>  $W_{\rm el}=2.97~cm^3$  $W_{pl} = 3.71 \text{ cm}^3$  $I = 4.29 \text{ cm}^4$ 

Material: EN 10219-S235JRH

rmissible load of base plate 80 reinforced																						
stancion I			in ca	se of	a siı	nulta	aneou	Perr usly a	nissi Ictin	ble v g hor	ertic: izont	al loa al loa	nd N∣ ad H	[kN] [kN]	for d	iffere	ent li	ft hei	ights	;		Perm. hori- zontal load
fcml	ŀ	l=0.	0	ŀ	l=1.	0	ŀ	l=2.0	0	ŀ	l = 3.	0	ŀ	<b>I</b> = 4.0	D	H	l=5.0	)	ŀ	l = 6.0	0	H [kN],
[]				,						Le	vel [I	n]										
	2.0	1.5	1.0	2.0	1.5	1.0	2.0	1.5	1.0	2.0	1.5	1.0	2.0	1.5	1.0	2.0	1.5	1.0	2.0	1.5	1.0	N = 0 KN
0	391	54²	69	391	54²	68	391	53	67	391	53	66	38	52	64	38	51	63	38	50	61	30.0
5	39 <sup>1</sup>	54²	68	39 <sup>1</sup>	53	66	38	52	64	38	50	62	37	49	59	35	47	56	34	44	53	8.9
10	39 <sup>1</sup>	53	67	38	52	64	38	49	57	36	47	50	34	43	43	31	40	36	28	34	25	5.2
15	39 <sup>1</sup>	53	65	38	50	55	36	46	46	33	40	36	29	29	25	-	-	-	-	-	-	3.7
20	38	51	60	37	47	48	33	38	36	28	26	23	-	-	-	-	-	-	-	-	-	2.8
25	38	50	55	35	41	41	30	28	27	-	-	-	-	-	-	-	-	-	-	-	-	2.3
30	37	47	50	33	35	35	7	20	19	-	-	-	_	-	-	-	-	-	-	-	-	2.0
35	36	42	45	28	29	29	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	1.7
40	34	38	40	23	24	24	-	-	-	-	-	-	_	-	-	-	-	-	-	-	-	1.5
45	32	33	35	13	16	19	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	1.3
51	27	28	29	5	6	7	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	1.2

The permissible vertical loads are calculated by application of the calculation model according to DIN EN 12811-1, para. 10.2.3.2. To consider the bending stiffness of the Allround standard, the effects from second-order theory and the maximum load-bearing capacity of the standards, birdcage scaffolding with modular dimension 2.57 x 2.57 m and different lift heights was considered.

(-) With this combination of spindle extension and horizontal load, the load-bearing capacity of the spindle is exceeded.

<sup>1</sup> Here the permissible vertical load of the standard at 2.0 m lift height is reached (39 kN) <sup>2</sup> Here the permissible vertical load of the standard at 1.5 m lift height is reached (54 kN)

#### 2.0 m lift height 70 60 50 [NN] N [KN] ——— H=2.0 kN ertical l 30 E 20 10 0 25 40 45 50 55 5 10 15 20 35 0 Extension Ia [cm]







→ Rolled thread:  $f_{y,k} = 280.0 \text{ N/mm}^2$ 

### BASE PLATE 60 SOLID LOADING TABLE



Equivalent section properties of the thread

 $A = 8.80 \text{ cm}^2$ 

 $W_{el} = 3.84 \text{ cm}^3$ 

 $W_{l}^{e^{-1}} = 4.79 \text{ cm}^{3}$  $I_{l}^{e^{-1}} = 6.51 \text{ cm}^{4}$ 

Material: EN 10025-2-S355J2  $\rightarrow$  Rolled thread:  $f_{v,k} = 360.0 \text{ N/mm}^2$ 

ermissible id																						
			in ca	se of	a sir	nulta	aneou	Perr usly a	nissi actin	ble v g hor	ertica izont	al loa al loa	id N∣ ad H	(kN) (kN)	for d	liffer	ent li	ft he	ights			Perm. hori- zontal load
xtension I	ŀ	1=0.0	0	H	1=1.	0	H	l=2.0	0	ŀ	<b>I</b> = 3.	0	ŀ	<b>l</b> = 4.0	0	ŀ	l=5.	0	ŀ	l = 6.0	)	H [kN],
[cm]										Le	vel [r	n]										when
	2.0	1.5	1.0	2.0	1.5	1.0	2.0	1.5	1.0	2.0	1.5	1.0	2.0	1.5	1.0	2.0	1.5	1.0	2.0	1.5	1.0	N = 0 kN
0	39 <sup>1</sup>	54²	69	39 <sup>1</sup>	54²	68	39 <sup>1</sup>	53	67	39 <sup>1</sup>	53	65	38	52	64	38	51	62	38	50	61	43.6
5	39 <sup>1</sup>	54²	68	39 <sup>1</sup>	53	66	38	52	64	38	50	61	37	48	59	36	47	56	35	45	53	14.1
10	39 <sup>1</sup>	53	67	38	52	64	38	49	61	36	47	57	35	45	53	33	42	49	32	40	44	8.4
15	39 <sup>1</sup>	53	66	38	50	61	36	47	57	35	43	52	33	41	46	29	38	40	26	34	31	6.0
20	38	51	64	37	48	58	35	44	52	31	41	46	28	36	39	-	-	29	_	-	-	4.7
25	38	50	61	36	45	54	33	41	47	28	37	39	-	-	-	-	-	-	-	-	-	3.8
30	37	48	57	34	43	50	29	38	41	11	15	27	-	-	-	-	-	-	-	-	-	3.2
35	36	45	53	30	40	44	20	27	31	-	-	-	-	-	-	-	-	-	-	-	-	2.8
39	34	42	48	27	35	36	12	13	18	-	-	-	-	-	-	-	-	-	_	-	-	2.5

The permissible vertical loads are calculated by application of the calculation model according to DIN EN 12811-1, para. 10.2.3.2. To consider the bending stiffness of the Allround standard, the effects from second-order theory and the maximum load-bearing capacity of the standards, birdcage scaffolding with modular dimension 2.57 x 2.57 m and different lift heights was considered.

(-) With this combination of spindle extension and horizontal load, the load-bearing capacity of the spindle is exceeded.

<sup>1</sup> Here the permissible vertical load of the standard at 2.0 m lift height is reached (39 kN) <sup>2</sup> Here the permissible vertical load of the standard at 1.5 m lift height is reached (54 kN)









## BASE PLATE 60 SWIVELING, REINFORCED LOADING TABLE



Equivalent section properties of the thread  $A = 4.71 \text{ cm}^2$   $W_{el} = 2.97 \text{ cm}^3$ 

- $W_{pl} = 3.71 \text{ cm}^3$  $I = 4.29 \text{ cm}^4$
- Material: EN 10219-S235JRH  $\rightarrow$  Rolled thread:  $f_{vk} = 280.0 \text{ N/mm}^2$

ermissible load of base plate 60 swiveling, reinforced																						
			in ca	se of	a siı	nulta	ineoi	Perr usly a	nissi Ictin	ble v g hor	ertica izont	al loa al loa	id N   ad H	[kN] [kN]	for d	liffer	ent li	ft hei	ights			Perm. hori- zontal load
xtension I <sub>a</sub>	H=0.0		H=0.0 H=1.0		H=2.0 H=3.0			H=4.0 H=5.0			H	H = 6.0		H [kN],								
fourl										Le	vel [r	n]										when
	2.0	1.5	1.0	2.0	1.5	1.0	2.0	1.5	1.0	2.0	1.5	1.0	2.0	1.5	1.0	2.0	1.5	1.0	2.0	1.5	1.0	N = 0 kN
0	39 <sup>1</sup>	44 <sup>3</sup>	44 <sup>3</sup>	38	44 <sup>3</sup>	44 <sup>3</sup>	37	44	44	36	44	44	35	44	44	34	44	44	32	38	44	14.3
5	38	44 <sup>3</sup>	44 <sup>3</sup>	37	44 <sup>3</sup>	44 <sup>3</sup>	35	44	44	33	39	41	28	30	33	21	22	23	11	11	12	6.7
10	37	44 <sup>3</sup>	44 <sup>3</sup>	35	43	44 <sup>3</sup>	29	32	35	20	21	23	8	8	8	-	-	-	-	-	-	4.3
15	36	44 <sup>3</sup>	44 <sup>3</sup>	29	34	37	19	20	22	5	5	6	-	-	-	-	-	-	-	-	-	3.2
20	33	39	43	23	25	28	10	10	11	-	-	-	-	-	-	-	-	-	-	-	-	2.5
25	29	32	36	17	19	20	2	2	3	-	-	-	-	-	-	-	-	-	-	-	-	2.1
30	25	27	30	12	13	14	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	1.8
31.5	23	26	29	11	12	13	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	1.7

The permissible vertical loads are calculated by application of the calculation model according to DIN EN 12811-1, para. 10.2.3.2. To consider the bending stiffness of the Allround standard, the effects from second-order theory and the maximum load-bearing capacity of the standards, birdcage scaffolding with modular dimension 2.57 x 2.57 m and different lift heights was considered.

(-) With this combination of spindle extension and horizontal load, the load-bearing capacity of the spindle is exceeded.

<sup>1</sup> Here the permissible vertical load of the standard at 2.0 m lift height is reached (39 kN)

<sup>3</sup> Here the load-bearing capacity of the M16 bolt is reached (interaction of bending and shear, permissible vertical load is 44 kN)









## ALLROUND BRACKETS





Permiss	ible load of Allro	und brackets, K 2	2000+ and LW								
					Bracket	0.73 m			Dracket 1 00 m		
		Bracket 0.39 m			without support		with support	Bracket 1.09 m with bracing ledger			
Bay length w [m]	perm. concentrated load on spigot [kN]	perm. uniformly dis- trubted load on bracket deck [kN/m²]	Load class*	perm. concentrated load on spigot [kN] perm. uniformly dis- trubted load Load class on bracket deck [kN/m <sup>2</sup> ]		Load class*	perm. concentrated load [kN]	perm. uniformly dis- trubted load on bracket deck [kN/m²]	Load class		
2.07		6.7	5		3.4		6		4.2		
2.57	2.6	5.2	4	2.2	2.6	3	5	5.2	3.3	3	
3.07		4.3	4		2.1		4		2.7		

Please note: The concentrated loads quoted and the uniformly distributed load on the bracket deck must not act simultaneous!!

The load classes quoted apply to the use of steel decks. The permissible loads quoted apply for decks double-sided. "Nominal load only, not a partial area load



## PLATFORM STAIR / COMFORT STAIR

## **INFORMATION ON APPLICATIONS**

The platform stair / comfort stair of aluminium ensure safer ascent and descent at the scaffolding.

Scaffolding users always have one hand free and can carry tools or work materials without any problem.

The platform stair / comfort stair is available for the bay lengths 2.57 m and 3.07 m, in U- and O-versions, and in the widths 0.64 m and 0.94 m. Stairs with width 0.64 m fit into 0.73 m wide scaffolding bays, and stairs with width 0.94 m into 1.09 m wide bays. With the platform stair / comfort stair, accesses of different types can be provided with different features, for example:

- 4-standard stairtower: either integrated into the scaffolding or as a separate access anchored to the building
- Unidirectional or alternating stair access
- Stair access "classic" with stair height of 2.0 m or modular stairtower with module / stair height of 2.21 m, consisting of modules that can be preassembled and moved by crane



End module Stair module Stair module Stair module Stair module with red. side protection Basis Module stairtower, alternating

To compensate for height differences in the ground, a variety of initial stairs in the heights 1.0 m, 1.20 m and 1.70 m are available. Matching outer, inner and continuous guardrails round off the range for aluminium platform stairs / comfort stairs.



The proven platform stair conforms to stair class A as per EN 12811-1.

• 10 risers with a stair height of 2.0 m



The comfort stair conforms to stair class B as per EN 12811-1. The comfort stair is based on the platform stair, but has a more comfortable step dimension plus a reinforced and hence stiffer step section and stringer section. All these characteristics enable much more pleasant ascent and descent, meaning the comfort stair is particularly suitable when greater heights have to be accessed.

• 9 risers with a stair height of 2.0 m

## LOADING TABLE

Stairs	Permissible area load on the entire area of the stair (on all steps and landings) [kN/m²]
All stairs of width 0.64 m	2.5*
All stairs of width 0.94 m	2.0*

\*The requirement of EN 12811-1:  $q_{perm} = 1.0 \text{ kN/m}^2$  is met.

# The steps and landings of the platform stair and of the comfort stair were verified for a permissible concentrated load of 1.5 kN according to the stipulations of EN 12811-1, 6.2.4 in addition to the permissible area load. Verification of the fatigue strength of the welded-on stair steps of aluminium was in accordance with the stipulations of EN 12810-1, 8.5.1 by tests as per EN 12810-2, Annex C.

## GAP-FREE TRANSITION TO SCAFFOLDING

Gap-free transition from the stair to the scaffolding can be provided with the aid of the gap ledger, 0.11 m or the telescoping U-gap deck. The construction depends on the width of the scaffolding bay and on the width of the scaffolding decks used.

Example: Gap-free transition to 0.73 m wide scaffolding with gap ledger, 0.11 m and 0.61 m wide scaffolding deck (Xtra-N deck, Robust deck, Stalu deck)





General view of transition

Detailed view

## STAIR STRINGERS

## **STAIR STRINGER 200**

Rectangular tube 60 x 50 x 2.0 mm Material: EN 10219-S235JRH

Live load of the stair stringer 200					
Length of steps [m]	Steel deck, one-sided perm. p [kN/m²]				
1.09	2.7				
1.29	2.2				
1.40	2.0				
1.57	1.7				
2.07	1.3				
2.57	1.0				

With the Allround construction stairtower 200, 12-standard, each stair is assembled from 2 separate U-stair stringers 200, and 32 cm wide steel decks used as steps.

Separate stringers and decks permit variable stair widths (1.09 m, 1.57 m, 2.07 m, 2.57 m). Weight and volume of the parts are kept low and the stair can be constructed completely from standard Layher Allround material.



Stair dimensions: Riser r = 20.0 cm; going g = 24.1 cm; overlap = 7.9 cm



## **STAIR STRINGER 500**

The stairtower 500 is intended for temporary stair structures with higher live loads. It is preferably used as a construction stairtower, e.g. for access to the site or as a road crossing not open to the public during construction work, but also at buildings as an additional escape stairtower. The stair steps are 32 cm wide steel decks. Under certain circumstances, the stairtower 500 can also be used for public access during construction work or as a mandatory escape stairtower.

#### Manufacture until 2012

Rectangular tube 100 x 50 x 3.6 mm Material: EN 10219-S235JRH

Live load of the stair stringer 500								
Length	perm. p on the steel decks [kN/m²]							
of steps [m]	Steel deck, one-sided	Steel deck, double-sided						
1.09	11.7	5.6						
1.40	9.0	4.3						
1.57	7.9	3.8						
2.07	5.9	2.8						
2.57	4.7	2.2						



Stair dimensions: Riser r = 20.0 cm; goinig g = 27.5 cm; overlap = 4.5 cm.

#### Manufacture starting 2012

Rectangular tube 100 x 50 x 2.5 mm Material: EN 10219-S355JRH

Live load of the stair stringer 500						
Length of stors	perm. p on the steel decks [kN/m²]					
[m]	Steel deck, one-sided	Steel deck, double-sided				
1.09	12.8	6.1				
1.40	9.8	4.7				
1.57	8.7	4.2				
2.07	6.5	3.1				
2.57	5.2	2.4				



Basis for dimensioning: EN 1993-1-1 Partial factors used:

$$\begin{split} \gamma_{\text{M0}} = & 1.0 \text{ in accordance with the recommendation of} \\ & \text{DIN EN 1993-1-1 and the stipulation in DIN EN} \\ & 1993-1-1\text{NA for cross-section verifications in} \\ & \text{which the internal forces were not determined} \\ & \text{according to second-order theory.} \end{split}$$

 $\gamma_{G}$  = 1.35 as per DIN EN 1990

$$\gamma_{F} = 1.5$$

### **STAIR STRINGER 750**

The stairtower 750 with child-safety guardrail is intended, in view of its riser dimensions, for both temporary and permanent stair structures in public areas. Typical applications are as road-crossings during building work, as stairs inside buildings for the duration of the construction work, as a mandatory escape stairtower or as a construction stairtower. The stair steps are 32 cm wide steel decks. For the events field, the stairtower 750 has a high load-bearing capacity, allowing it to be used for accessing stands and stages.



Stair dimensions: Riser r = 16.6 cm; goinig g = 31.0 cm; overlap = 1.0 cm.

#### Manufacture until 2012

Rectangular tube 120 x 50 x 4.0 mm Material: EN 10219-S235JRH

Live load of the stair stringer 750							
Length	perm. p on the steel decks [kN/m²]						
of steps [m]	Steel deck, one-sided	Steel deck, double-sided					
1.09	17.5	8.4					
1.40	13.4	6.5					
1.57	11.9	5.7					
2.07	8.9	4.3					
2.57	7.1	3.4					

#### Manufacture starting 2012

Rectangular tube 120 x 50 x 3.0 mm Material: EN 10219-S355JRH

Live load of	the stair string	er 750				
Length of stors	perm. p on the steel decks [kN/m²]					
[m]	Steel deck, one-sided	Steel deck, double-sided				
1.09	20.5	9.9				
1.40	15.7	7.6				
1.57	14.0	6.8				
2.07	10.5	5.0				
2.57	7.5*/8.4**	4.0				

\* Steel decks of earlier design \*\* Steel decks LW



Basis for dimensioning: EN 1993-1-1 Partial factors used:

- $$\begin{split} \gamma_{\text{M0}} = 1.0 \text{ in accordance with the recommendation of} \\ \text{DIN EN 1993-1-1 and the stipulation in DIN EN} \\ 1993-1-1\text{NA for cross-section verifications in} \\ \text{which the internal forces were not determined} \\ \text{according to second-order theory.} \end{split}$$
- $\gamma_{_{G}}~=1.35$  as per DIN EN 1990
- $\gamma_{_F} \quad = 1.5$

## **CONNECTION VALUES AS PER GERMAN APPROVAL** DESIGN RESISTANCES IN ALLROUND LEDGER AND DIAGONAL BRACE CONNECTION

### 7-8.22-64.1: ALL ROUND ALUMINIUM

#### **Bending moment**



a) If normal force Nst [kN] in the standard  $\leq$  is 45 kN: M<sub>y, Rd</sub> =  $\pm$  60 kNcm b) If normal force Nst [kN] in the standard > is 45 kN:  $M_{y, Rd} = \pm \left[\frac{60 \text{ x} (63 - \text{Nst})}{18}\right] \text{ [kNcm]}$ 

Normal force



 $N_{\text{Rd}} = \pm \; 18.5 \; kN$ 



 $N_{V, Rd} = \pm 9.0 \text{ kN}$ 

Vertical shear force



Horizontal shear force



 $V_{y, Rd} = \pm 6.0 \text{ kN}$ 

#### a) Vertical shear force single connection $V_{Z, Rd} = \pm 18.1 \text{ kN}$

b) Vertical shear force per rosette  $\sum V_{Z, Rd} = 46.4 \text{ kN}$ 

## LOADING TABLES FOR ALLROUND ALUMINIUM ALL SPECIFIED LOADS ARE SAFE WORKING LOADS.

Inner standard 2.0 m lift height							
Bay width [m]	0.73		1.09	1.57	2.07	2.57	3.07
Diagonal bracing	А	В	А, В	А, В	А, В	В	В
Permissible vertical load V <sub>1</sub> [kN]	15.5	13.7	14.7	14.6	14.4	14.2	14.0

1.57	2 07	2 5 7	0.07
	2.07	2.37	3.07
В	В	В	В
12.5	12.1	11.9	11.7
	B 12.5	B B 12.5 12.1	B         B         B           12.5         12.1         11.9

Permissible load of aluminium U-ledger (U), U-ledger reinforced (U-V)								
Ledger type and length [m]	0.73 (U)	1.09 (U-V)	1.40 (U-V)					
Uniformly distributed line load (q) [kN/m]	17.8	10.7	8.4					
Concentrated load (P) in bay centre [kN]	5.9	7.2	5.7					



Uniformly distributed line load (q) [kN/m] 18.7 7.4

Concentrated load (P) in bay centre [kN] 6.3 4.5

Permissible load of Alu ledger

Bay width [m]

Permissible load of Alu-U-lattice beam				
Bay width [m]	2.57	3.07	4.14	5.14
Uniformly distributed line load (q) [kN/m]*	7.7	6.0	4.1	3.2
Concentrated load (P) in bay centre [kN]*	6.7	11.4	8.9	8.0

0.73 1.09 1.40 1.57

3.9 2.9

3.4

2.9

2.07

2.0

2.57 3.07

1.5 1.2

1.5 0.9 0.6

### Permissible load of aluminium Allround standards





Diagonal bracing A: 1 diagonal brace for 2 bays





A = Outer standard I = Inner standard

A = Outer standard I = Inner standard

\* Completely covered with

View

		scaffol	ding decks	
	Permissible load of Alu U-bridging ledger			
	Bay width [m]	1.57	2.07	
<u>m</u>	Uniformly distributed line load (q) [kN/m]*	6.9	3.7	
	Concentrated load (P) in bay centre [kN]*	6.2	2.3	

## SCAFFOLDING DECKS

## LOAD CLASSES AND USE IN PROTECTIVE SCAFFOLD<sup>1</sup> AND ROOF EDGE PROTECTION SCAFFOLD<sup>2</sup> ACCORDING TO GERMAN APPROVAL

Steel decks														
Load class	U-	and O-ste Ref. I	el decks 0 No. 3802, 1	.32 m wide 3812, 388	e (without 3, 3844, 3	t web hole 861, 3862	s, T4/T9, 2, 3890	LW)	S	teel decks Ref. No. 3	0.19 m wic 8801, 3863	le,	Steel acc Art. No	ess deck, o. 3813
EN 12011-1	0.73	1.09	1.40	1.57	2.07	2.57	3.07	4.14	1.57	2.07	2.57	3.07	2.07	2.57
perm. q up to and including T4/T	27.6	25.2	10.7	17 5	11.4	7.5	5.0	2.0	177	11 /	75	5.0		
[kN/m <sup>2</sup> ] LW	- 37.0	20.0	13.7	17.5	13.3	9.3	6.5	2.0	17.7	11.4	7.J	J.U	_	_
1	•	•	•	•	•	•	•	•	•	•	•	•	•	•
2	•	•	•	•	•	•	•	•	•	•	•	•	•	•
3	•	•	•	•	•	•	•	•	•	•	•	•	•	•
4	•	•	•	•	•	•	•	-	•	•	•	•	•	•
5	•	•	•	•	•	•	-	-	•	•	•	-	-	-
6	•	•	•	•	•	-	-	-	•	•	-	-	-	-
Protective scaffold and roof edge protection scaffold	•	•	•	•	•	•	•	•	•	•	•	•	•	•

Robust decks, Xtra-N decks												
Load class	Rob Xtra	ust deck a-N deck	: 0.61 m , 0.61 m	wide, Re wide, Re	f. No. 38 f. No. 38	35 66	Robust de	eck 0.32 m v	vide, Ref. N	o. 3836	Robust access decks Xtra-N acces	s, Ref. No. 3838, 3858, 3859, s deck, Ref. No. 3869
EN IZOTI-I	0.73	1.09	1.57	2.07	2.57	3.07	1.57	2.07	2.57	3.07	2.57	3.07
1	•	•	•	•	•	•	•	•	•	•	•	•
2	•	•	•	•	•	•	•	•	•	•	•	•
3	•	•	•	•	•	•	•	•	•	•	•	•
4	-	-	-	-	-	-	•	•	•	-	-	-
5	-	-	-	-	-	-	•	•	-	-	-	-
6	-	-	-	-	-	-	•	-	-	-	-	-
Protective scaffold and roof edge protection scaffold	•	•	•	•	•	•	•	•	•	•	•	•

#### Stalu decks

Statu uccks													
Load class	S	Stalu decks Ref. No. 3	0.61 m wid 850, 3867	e,		Stalu F	deck 0.32 m Ref. No. 385	n wide, 56		:	Stalu deck ( Ref. N	).19 m wide o. 3857	
EN 12011-1	1.57	2.07	2.57	3.07	1.57	2.07	2.57	3.07	4.14	1.57	2.07	2.57	3.07
1	•	•	•	•	•	•	•	•	•	•	•	•	•
2	•	•	•	•	•	•	•	•	•	•	•	•	•
3	•	•	•	•	•	•	•	•	•	•	•	•	•
4	•	•	•	•	•	•	•	•	-	•	•	•	•
5	•	•	•	-	•	•	•	-	-	•	•	•	-
6	•	•	-	-	•	•	-	-	-	•	•	-	-
Protective scaffold and roof edge protection scaffold	•	•	•	•	•	•	•	•	•	•	•	•	•

Alu decks														
		Al	u deck O	.32 m wi	de,		Alu d	eck 0.19 m	wide,		Alu a	ccess deck	s, Ref. No	see below
Load class			Ref. N	o. 3803			R	ef. No. 382	24		3851	, 3852		3851.100, 3871.100
EN IZOII-I	0.73	1.09	1.57	2.07	2.57	3.07	1.57	2.07	2.57	1.57	2.07	2.57	2.57	1.00
1	•	•	•	•	•	•	•	•	•	•	•	•	•	•
2	•	•	•	•	•	•	•	•	•	•	•	•	•	•
3	•	•	•	•	•	•	•	•	•	•	•	•	•	•
4	•	•	•	•	•	-	•	•	•	-	-	-	-	-
5	•	•	•	•	-	-	•	•	-	-	-	-	-	-
6	•	•	•	-	-	-	•	-	-	-	-	-	-	-
Protective scaffold and roof edge protection scaffold	•	•	•	•	•	•	•	•	•	•	•	•	•	•

 Protective scaffold: scaffolding arresting the fall of a person
 Roof edge protection scaffold: scaffolding, including a protective wall, arresting the fall of a person sliding down a sloping surface • Approved for use in the load class/approved for use in the standard brick guard and roof brick guard. Suitability for use in protective scaffold and roof edge protection scaffold has been verified by drop tests as per EN 12810-2, Annex B. (-) not approved for this load class.

## **USE AS FACADE SCAFFOLDING**



No vertical diagonal braces are required in the standard assembly according to the German approval. Assembly variants diverging from the standard version must be verified by structural analysis. Vertical diagonal braces may be required here depending on the height of the scaffolding, on the anchoring configuration, from the presence of cladding, on the load and on the scaffolding width. Experience suggests that assembly variants other than the standard version can be implemented with vertical diagonal braces in every 5th bay.

In scaffolding levels without decks, O-horizontal diagonal braces must be installed in every 5th bay and longitudinal ledgers on the inside and outside. This also applies for scaffolding levels with wooden planks.

Use as facat	ie scalioi	ung							
Load class EN 12811-1	Nominal load q <sub>1</sub> [kN/m²]	Partial a [kN/m²]	area load q <sub>2</sub> Partial area A <sub>c</sub> <sup>1)</sup> [m²]	Concen- trated load F <sub>1</sub> [kN]	Application	Scaffold- ing width b [m]	Scaffold- ing bay length [m]	Support ledger	Deck type
1	0.75	Not requ	iired	1.5	Inspection purposes, Working with light tools, without building material storage.	0.73	3.07	U-ledger LW, O-ledger LW, U-ledger, O-ledger	All scaffolding decks
2	1.5	Not requ	iired	1.5	Inspection work, work with materials	0.73	3.07	U-ledger LW, O-ledger LW,	All scaffolding decks
3	2.0	Not requ	iired	1.5	that are consumed immediately, e.g. painting, stone cleaning, grouting, plastering etc.			U-ledger, O-ledger	
4	3.0	5.0	0.4 x A <sup>2)</sup>	3.0	Bricklaying, attachment of	1.09	3.07	U-ledger LW, O-ledger LW, U-ledger reinforced	Steel decks, Stalu decks
					prefabricated concrete parts, plastering etc.	1.40	3.07	U-ledger LW, U-ledger LW reinforced, O-ledger LW reinforced	
						1.40	2.57	U-ledger LW, U-ledger rein- forced, O-ledger LW reinforced	Steel decks, Stalu decks, Robust decks (0.32 m wide),
						1.09	2.07	O-ledger LW, O-ledger, U-led- ger LW	Aluminium decks (0.32 m wide), Xtra-N decks (0.32 m
						1.09	2.57	Ú-ledger LW, O-ledger LW, O-ledger reinforced, U-ledger reinforced	wide)
						1.57	3.07	U-ledger LW reinforced, U-bridging ledger, O-bridging ledger, O-ledger I W reinforced	Steel decks, Stalu decks
5	4.5	7.5	0.4 x A <sup>2)</sup>	3.0		1.09	2.07	O-ledger LW, U-ledger rein- forced, U-ledger LW	Steel decks, Stalu decks, Robust decks (0.32 m wide),
						1.40	2.07	U-ledger LW reinforced, O-ledger LW reinforced	Aluminium decks (0.32 m wide), Xtra-N decks (0.32 m
						1.40	1.57	U-ledger, reinforced	wide)
						1.57	2.07	U-ledger LW reinforced, U-bridging ledger, O-bridging ledger, O-ledger LW reinforced	
						1.57	2.57	U-ledger LW reinforced, U-bridging ledger, O-bridging ledger, O-ledger LW reinforced	Steel decks, Stalu decks
						2.07	2.07	U-ledger LW reinforced	
6	6.0	10.0	0.5 x A <sup>2)</sup>	3.0	Heavy bricklaying or natural stone- work. Storage of a large quantity of	1.09	1.57	O-ledger LW, U-ledger rein- forced, U-ledger LW	Steel decks and Stalu decks up to scaffolding bay length
					building materials or components	1.09	2.07	U-ledger reinforced, U-ledger LW, O-ledger LW reinforced	2.07 m, Xtra-N decks (0.32 m wide) and Robust
						1.40	2.07	U-ledger LW reinforced, O-ledger LW reinforced	decks (0.32 m wide) up to scaffolding bay length 1.57 m
						1.57	1.57	U-ledger LW reinforced, U-bridging ledger, O-ledger LW reinforced	
						1 57	2.07	II-ledger I W reinforced	

 $^{1})$  Ac = partial area  $^{2})$  A = Platform area of a scaffolding bay

pan in [m] for scaf	folding decks mad	le of wooden plan	ks or boards (acco	ording to Tab. 2, D	IN 4420-3:2006)
Board or plank		Board	or plank thickness	s [mm]	
width [mm]	30	35	40	45	50
200	1.25	1.50	1.75	2.25	2.50
240) and 280)	1.25	1.75	2.25	2.50	2.75
200	1.25	1.50	1.75	2.25	2.50
240) and 280)	1.25	1.75	2.00	2.25	2.50
200, 240, 280	1.25	1.25	1.50	1.75	2.00
200, 240, 280	1.00	1.25	1.25	1.50	1.75
	pan in [m] for scaf           Board or plank           width [mm]           200           240) and 280)           200           240) and 280)           200           240) and 280)           200, 240, 280           200, 240, 280           200, 240, 280	pan in [m] for scaffolding decks mad           Board or plank           width [mm]           30           200           1.25           240) and 280)           1.25           240) and 280)           1.25           240) and 280)           1.25           200           200           1.25           200, 240, 280           1.00	pan in [m] for scaffolding decks made of wooden plank           Board or plank         Board           width [mm]         30         35           200         1.25         1.50           240) and 280)         1.25         1.75           200         1.25         1.50           240) and 280)         1.25         1.50           240) and 280)         1.25         1.75           200, 240, 280         1.25         1.25	pan in [m] for scaffolding decks made of wooden planks or boards (accord)           Board or plank width [mm]         30         35         40           200         1.25         1.50         1.75           240) and 280)         1.25         1.75         2.25           200         1.25         1.50         1.75           240) and 280)         1.25         1.50         1.75           240) and 280)         1.25         1.50         1.75           200, 240, 280         1.25         1.25         1.50	Pan in [m] for scaffolding decks made of wooden planks or boards (according to Tab. 2, D           Board or plank         Board or plank         Board or plank thickness         mm]           30         35         40         45           200         1.25         1.50         1.75         2.25           240) and 280)         1.25         1.75         2.25         2.50           200         1.25         1.75         2.25         2.50           200         1.25         1.75         2.00         2.25           240) and 280)         1.25         1.75         2.00         2.25           240) and 280)         1.25         1.75         2.00         2.25           200, 240, 280         1.25         1.25         1.50         1.75           200, 240, 280         1.00         1.25         1.25         1.50

Select the Layher scaffolding decks in accordance with the required load class (see previous page)

For the use of wooden planks and boards in safety decking, arresting the fall of persons or objects, the information according to Tab. 2 DIN 4420-1:2004 applies.

## LAYHER STEEL PLANK

The steel plank is used for closing larger openings in the deck level.





### LOAD CLASSES AND PERMISSIBLE AREA LOADS

Steel plank		Load class as per FN 12811-1	Permissible area load (on the entire area						
Width [cm]	Length [m]		of the steel plank) [kN/m²]						
	1.0	6	10.0						
20	1.5	-							
20	2.0	5	7.5						
	2.5	3	2.0						
	1.0	C	10.0						
20	1.5	0	10.0						
30	2.0	5	7.5						
	2.5	3	2.0						

If at least 2 steel planks are adjacent to one another, they may also be used in protective scaffold and roof edge protection scaffold.

This has been verified by drop tests as per EN 12810-2, Annex B.

Compared with the wooden plank, the steel plank is durable, non-inflammable, non-slip and also lower-weight. Steel planks are available in the widths 20 cm and 30 cm. The support length must be at least 10 cm at every support.



## **CLOSING OF GAPS IN DECK LEVELS**



## STEEL GAP SHEET

For closing gaps between steel decks or aluminium decks, U or O versions, the steel gap sheet is used. The steel gap sheet is secured with short locking screws (blue) against slipping and lifting out. The width of the gap sheet is 32 cm.

#### According to German approval

Largest permissible gap width: 22 cm Load class 6 as per EN 12811-1 Permissible area load: 10 kN/m<sup>2</sup> (on the entire area of the gap sheet) Load transfer in transverse direction

## U-GAP SHEET, WITH HOOKS

The U-gap sheet with hooks was designed for closing gaps between unperforated U-scaffolding decks (Robust decks, Xtra-N decks, Stalu decks). Securing is achieved with the built-in Allround lift-off preventer. The width of the U-gap sheet is 32 cm.

Largest permissible gap width: 22 cm Load class 6 as per EN 12811-1 Permissible area load: 10 kN/m<sup>2</sup> (on the entire area of the gap sheet)

### **TELESCOPING U-GAP DECK**

The telescoping U-gap deck permits the closing of 40 mm to 255 mm wide gaps. Infinite adjustment to the gap dimension in question. Fixing by two integrated screws workable from above. Precisely fitting decking over the rosette even with the system ledger installed. Braces the decks in the scaffolding bay, securing them against inadvertent shifting. Load transfer in longitudinal direction.

#### According to German approval

Length of telescoping gap deck [m]	Load class according to EN 12811-1	Permissible area load (on the entire area of the gap deck) [kN/m²]
≤ 2.07	6	10.0
2.57	5	7.5
3.07	4	5.0

### U-STEEL DECK

The cap of the 32 cm wide U-steel deck is optimised in its shape to permit precisely fitting decking over the rosette.

For load classes and permissible area loads see table of scaffolding decks.







### 0- / U-GAP LEDGER 0.11 M WIDE

The 0.11 m wide gap ledger is used to close the gap between two scaffolding bays or between scaffolding bay and projection. The 0.11 m wide gap ledger is also needed at the start and end of a stair for connection to the stair landing.

#### According to German approval (both tables)

Load class and permissible area load of gap ledger LW 0.11 m									
Length of gap ledger LW [m]	Load class according to EN 12811-1	Permissible area load (on the entire area of the gap ledger) [kN/m²]							
≤ 3.07	6	10.0							
Load class and permissible	area load of gap ledger	<sup>r</sup> K2000+ 0.11 m							
Length of gap ledger K2000+ [m]	Load class according to EN 12811-1	Permissible area load (on the entire area of the gap ledger) [kN/m²]							
≤ 2.07	6	10.0							
2.57	5	7.5							
3.07	4	5.0							

## TYPES OF DECK LEVELS IN ALLROUND SCAFFOLDING



 $^{\ast}$  Ledger = support ledger on which the decks are laid

## LOAD CLASSES OF DECK LEVELS IN ALLROUND SCAFFOLDING

Load class	ses for deck	levels in	Allround	Scaff	ffolding																						
Ledger	Ledger	Ledger	perm.	Stee	l deck	ts on t	the le	dger o	n one	side						Stee	el deck	s on 1	he le	dger o	n botl	n side	s				
connec- tion to	type	length [m]	line load of	pern	nissibl	le load	d clas	s with	deck	lengt	h [m]					perr	nissibl	e loa	d clas	s with	deck	lengt	h [m]				
standard/			ledger	1.57			2.07	1		2.57			3.07			1.57	/		2.07	1		2.57			3.07		
type			[kN/m]	NL	6 m²	PL	NL	6 m²	PL	NL	6 m²	PL	NL	6 m²	PL	NL	6 m²	PL	NL	6 m²	PL	NL	6 m²	PL	NL	6 m²	PL
s	0 ledger	0.73	22.07	6	6	6	6	6	6	6	6	5+	5+	5+	4++	6	6	6	6	6	6	6	6	5+	5+	5+	4+
dard  +	K2000+	1.09	10.44	6	6	6	6	6	6	6	6	5	5+	5+	4+	6	6	5	5	5	4	4	4	4	4	4	-
stan 2000 iant		1.40	6.54 5.26	6	6	6	6 5	6	5	5	5	4	4	4	4	4	4	4	3	3	-	3	3	-	2	3	-
N, K N, K		2.07	3.12	0 	0 4	- 5 - 4	ี่ ว ว	3	4	4 3	4	4	4	4	_	4	4	_	ა 1	3 1	_	 1	2 1	_	 1	2 1	_
ULV LV		2.57	2.06	3	3	-	2	2	_	1	1	_	1	1	_	1	1	_	1	1	_		_	_	<u> </u>	_	_
4		3.07	1.46	2	2	-	1	1	_	1	1	_	- -	_	_	-	-	_	-	-	_	_	-	_	_	_	_
(0	0-ledger	0.73	29.24	6	6	6	6	6	6	6	6	5+	5+	5+	4++	6	6	6	6	6	6	6	6	5+	5+	5+	4++
lard:	LW	1.09	14.09	6	6	6	6	6	6	6	6	5+	5+	5+	4++	6	6	6	6	6	5	5	5	5	4	4	4
tanc 10+ ant		1.40	8.76	6	6	6	6	6	6	6	6	5	5	5	4+	5	5	5	4	4	4	4	4	-	3	3	-
nd s <200 Vari		1.57	7.03	6	6	6	6	6	5	5	5	5	4	4	4	4	4	4	4	4	-	3	3	-	3	3	-
Irou or		2.07	4.09	5	5	5	4	4	4	3	3	-	3	3	-	3	3	-	2	2	-	1	1	-	1	1	-
A		2.57	2.33 1.48	3 2	3	_	 1	3 1	_	<u> </u>	1	_			_		-	_			_	_	_	_	_	_	_
>	0-ledger	0.73	29.24	2 6	6	6	6	6	6	6	6	5+		5+	4++	6	6	6	6	6	6	6	6	5+	5+	5+	4++
s LV	LW	1.09	14.09	6	6	6	6	6	6	6	6	5+	5+	5+	4++	6	6	6	6	6	5	5	5	5	4	4	4
dard		1.40	8.76	6	6	6	6	6	6	6	6	5	5	5	4+	5	5	5	4	4	4	4	4	_	3	3	-
itano		1.57	7.03	6	6	6	6	6	5	5	5	5	4	4	4	4	4	4	4	4	-	3	3	-	3	3	-
s pu		2.07	4.09	5	5	5	4	4	4	3	3	-	3	3	-	3	3	-	2	2	-	1	1	-	1	1	-
Ilrou		2.57	2.65	4	4	-	3	3	-	2	2	-	2	2	-	1	2	-	1	1	-	1	1	-	_	-	-
∢	0 ladaan	3.07	1.85	3	3	-	2	2	-	1	1	- F+	1	1	— 4++	1	1	-	-	-	-	-	-	- F+	- F+	- F+	
ant II	0-ledger reinforced	1.09	Z1.8Z	b	b	b	b	b	b	b	b	5.	5⁺	5.	4++	б	0	b	b	b	b	b	b	5.	ວ*	5+	4+
Vari	0-bridg-	1.57	14.46	6	6	6	6	6	6	6	6	5	5+	5+	4++	6	6	6	6	6	5	5	5	5	5	5	4
+	ing ledger	2.07	8.63	6	6	6	6	6	6	6	6	5	5	5	4+	5	5	5	4	4	4	4	4	-	3	4	-
000		2.57	5.37	6	6	5	5	5	5	4	4	4	4	4	4	4	4	-	3	3	-	2	3	-	2	2	-
γ, K2		3.07	3.53	4	4	4	4	4	-	3	3	-	3	3	-	3	3	-	1	2	-	1	1	-	1	1	-
s LV	U-ledger	0.73	19.01	6	6	6	6	6	6	6	6	5+ E+	5⁺ ნ÷	5+ 5+	4++	6 6	6	6	6	6	6	6	6	5	5† F	5+ E+	4+
dard	u-leager, reinforced	1.09	17.34	0 6	6	6	0	6	6	0 6	0 6	5+	<u>ე</u> . 5+	5+	4· //+	0 6	6	5	0 5	5	0	0 /	0	с Л	с Л	о. Л	4.
stan	U-	1.57	15.16	6	6	6	6	6	6	6	6	5+	5+ 5+	5+	4++	6	6	6	6	6	5	5	5	5	5	5	4
pur	bridging	2.07	8.65	6	6	6	6	6	6	6	6	5	5	5	4+	5	5	5	4	4	4	4	4	_	3	4	_
Illrou	ledger	2.57	5.12	6	6	5	5	5	4	4	4	4	4	4	-	4	4	-	3	3	-	2	3	-	1	2	-
A		3.07	3.59	4	4	4	4	4	-	3	3	-	3	3	-	3	3	-	2	2	-		1	-	1	1	-
	U-ledger	1.09	17.55	6	6	6	6	6	6	6	6	5+	5+	5+	4++	6	6	6	6	6	6	6	6	5	5	5	4+
+	LW	1.40	10.84	6	6	6	6	6	6	6	6	5+	5+	5+	4+	6	6	5	5	5	5	4	4	4	4	4	4
200(	U-ledger	1.40	19.80	6 6	6	6	6	6 6	6	<u>b</u>	b	5⁺ 5+	_ე⊤ ნ+	5⁺ 5+	4++	6 6	6	6	<u>b</u>	b	6	6 6	6	5	_5*5	5⁺ 5+	4+
or K	forced	2.07	17.70	0 6	6	6	6	6	6	6	6	5+	5+	5+	4 ···	6	6	6	6	6	5	<u> </u>	5	Л		5	4
L M = I		2.07	8 40	6	6	6	6	6	6	6	6	5	5	5	4+	5	5	5	4	4	4	4	4	-	3	4	-
ariar		3.07	5.00	6	6	5	5	5	4	4	4	4	4	4	_	3	4	_	3	3	_	2	3	_	1	2	_
anda or Vá	0-ledger	1.09	21.40	6	6	6	6	6	6	6	6	5+	5+	5+	4++	6	6	6	6	6	6	6	6	5	5+	5+	4+
d sta	LW rein-	1.40	17.10	6	6	6	6	6	6	6	6	5+	5+	5+	4++	6	6	6	6	6	6	6	6	5	5	5	4+
uno.	forced	1.57	16.10	6	6	6	6	6	6	6	6	5+	5+	5+	4++	6	6	6	6	6	5	6	6	5	5	5	4+
Allr		2.07	11.10	6	6	6	6	6	6	6	6	5+	5+	5+	4+	6	6	5	5	5	5	4	5	4	4	4	4
		2.57	8.50	6	6	6	6	6	6	6	6	5	5	5	4	5	5	5	4	4	4	4	4	-	3	4	-
		3.07	0.00	0	0	5	2	5	5	- 4	5	4	- 4	4	4	- 4	4	4	3	4	_	- 3	3	_	Ζ	3	_

## APPLICATION OF THE TABLE FOR LOAD CLASSES OF DECK LEVELS IN ALLROUND SCAFFOLDING

#### Designation Explanation

### Sketch or example



Load

class 1

2

3

4

5

6

Associated partial

area load q2 [kN/m²]

5.00

7.50

10.00

Load class	Associated nominal load q <sub>1</sub> [kN/m <sup>2</sup> ]
1	0.75
2	1.50
3	2.00
4	3.00
5	4.50
6	6.00

Steel decks on one side:

Steel decks on both sides:



TL

NL

Partial area load q<sub>2</sub> as per EN 12811-1, Table 3 acting on 40% or 50% of the platform area of each scaffolding bay:

0.4

0.4

0.5

Steel decks on one side:

Steel decks on both sides:



 $q_2$  acts on 50 % of the deck area

of each scaffolding bay



 $w_1, \ell = axis dimensions of upright$ b" = entire deck width in scaffolding bay

6 m<sup>2</sup>

Limiting of the nominal load to an area of 6 m<sup>2</sup> as per EN 12811-1, 6.2.2.6: The load on the supporting components of a birdcage scaffolding may be calculated by assuming that the nominal load q, acts on an area of 6 m<sup>2</sup> in combination with a load of 0.75 kN/m<sup>2</sup> over the remaining area.

The load area of 6 m<sup>2</sup> is arranged such that it has for the ledger the least favourable effect.

The load-bearing capacity of the steel decks matches the load class indicated in front of the symbol "+". The load-bearing capacity of the ledger is one load class higher.

The load-bearing capacity of the steel decks matches the load class indicated in front of the symbol "+ +". The load-bearing capacity of the ledger is two load classes higher.

Steel decks on one side:

NL

5+

Steel decks on both sides:



The steel decks can support the nominal load of load class 5 (4.5 kN/m<sup>2</sup>). The load-bearing capacity of the ledger matches the nominal load of load class 6 (6.0 kN/m<sup>2</sup>).

6 m <sup>2</sup>	
5*	The steel decks can support the nominal load of the load class 5 $(4.5 \text{ kN/m}^2)$ on an area of 6 m <sup>2</sup> plus $0.75 \text{ kN/m}^2$ on the remaining area. The load-bearing capacity of the ledger matches the nominal load of load class 6 $(6.0 \text{ kN/m}^2)$ on an area of 6 m <sup>2</sup> plus $0.75 \text{ kN/m}^2$ on the remaining area.
PL	
4**	The steel decks can support the partial area load of load class 4 ( $5.0 \text{ kN/m}^2$ on 40% of the deck area of each scaffolding bay). The load-bearing capacity of the ledger matches the partial area load of load class 6 ( $10.0 \text{ kN/m}^2$ on 50% of the deck area of each scaffolding bay).

## TRANSFER OF TENSION OF STANDARD JOINT



Connecting means: Hinged pins or special bolts M12-8.8 with nut

Permissible tension force of Allround Standard LW with integrated spigot [kN]					
	Number	Standard above			
Standard below	of connecting means	Allround LW	Allround K2000+ or Variant II		
	1	36.4	29.5		
Allround LW with integrally cast spigot	2	69.3	59.0		



Connecting means: Hinged pins or special bolts M12-8.8 with nut

Permissible tension force of Allround Standard LW with bolt-in spigot [kN]					
0	Number of	Standard above			
Standard below, without spigot	connecting means above/below	Allround LW	Allround K2000+ or Variant II		
Alles	1/1	29.5			
AIIIOUIIU LVV	2/2	56.1			
Allround K2000+	1/1	32.6 29.5			
or Variant II	2/2	56.1	56.1		

Permissible tension force of Allround Standard Aluminium with bolt-in spigot [kN]			
Standard below, without spigot	Number of connecting means above/below	Standard above: Allround Aluminium	
Allround Aluminium	2/2	42.2	

Connecting means: Hinged pin or special bolt M12-8.8 with nut or locking pin red, dia 11 mm

#### Permissible tension force of Allround Standard with pressed-in spigot, K2000+ and Variant II:

6.7 kN

## SUSPENDED SCAFFOLDING WITH LAYHER LATTICE BEAM 450 STEEL

Load class 3 EN 12811-1 ( $q_1 = 2.0 \text{ kN/m}^2$  over 6 m<sup>2</sup>, remaining area with 0.75 kN/m<sup>2</sup>) Deck: Steel deck; Aluminium deck; Stalu deck; Robust deck, wooden planks d = 4.5 cm

Suspended scaffolding with Layher lattice beam 450 steel					
		Maximum suspension force <sup>1</sup> in [kN]			
Span a of lattice beams [m]	Top chord bracing interval b [m]	Single-bay Multiple-bay bear beam			
4.0	2.0	9.6	20.2		
6.0	1.5	13.4	27.9		

**Remarks:** Decks must be secured against lift-out. The suspended scaffolding must be secured to prevent swinging.

GTR

<sup>1</sup> The values are working loads.

Detail A — Version: Wooden planks.



#### Detail A1 — Version: Aluminium U-profile with half-couplers for decks





GTR = Lattice beam

- NK = Double coupler
- R = Tube
- VK = Supplementary coupler

Multiple-bay beam



View A GTR Diagonal tube in every 5th bay GTR C Single-bay beam



Bracing of top chord



#### Bracing of bottom chord







#### Bracing of bottom chord



## ALUMINIUM FLEXBEAM



**FlexBeam** Weight 30.0 kg – 70.0 kg Ref. No. 2657.300, 2657.400, 2657.500, 2657.600, 2657.700







**FlexBeam standard connector** Weight 6.6 kg Ref. No. 2657.080

FlexBeam tie rod adapter Weight 5.7 kg Ref. No. 2657.050

**FlexBeam standard adapter male** Weight 1.7 kg Ref. No. 2657.060

FlexBeam standard adapter female Weight 2.9 kg Ref. No. 2657.070

**FlexBeam suspension shoe** Weight 9.3 kg Ref. No. 2657.040

**FlexBeam anchor plate** Weight 12.0 kg Ref. No. 2657.030



**FlexBeam anchor plate tube** Weight 1.3 kg Ref. No. 2657.020

FlexBeam timber beam support Weight 3.4 kg Ref. No. 2657.090

FlexBeam lift-off preventer Weight 3.3 kg Ref. No. 2657.100



FlexBeam lift-off preventer lock Weight 8.1 kg, 50 pcs. Ref. No. 2657.110



**FlexBeam lift-off preventer bolt** Weight 2.8 kg, 20 pcs. Ref. No. 2657.120



FlexBeam rosette adapter Weight 2.7 kg Ref. No. 2657.130



**Bolt, dia. 20 x 113 mm** Weight 3.0 kg, 10 pcs. Ref. No. 2646.280



Securing pin, dia. 4 mm Weight 1.5 kg, 50 pcs. Ref. No. 5905.001





## BRIDGE REPAIR AS EXAMPLE OF APPLICATION

The aluminium FlexBeam has a high load capacity but low height. This means that standing and suspended surface scaffolding can be assembled even more economically. Extension using standard Allround components in the system dimension – for example side protection – is possible. Interchangeable U-ledgers also ensure high flexibility when positioning the suspension shoe.

For further information please refer to the Instructions for Assembly and Use.



The table values  $b_1$  were calculated on the assumption of a four metre high and fully covered protective wall with a one-metre projection beyond the bridge structure. For the protective wall the working wind load w was assumed, calculated with a dynamic pressure of 0.2 kN/m<sup>2</sup>.

 $w = 1.3 \text{ x} 0.2 \text{ kN/m}^2 = 0.26 \text{ kN/m}^2$ 

The distance  $b_{0}$  should be at least 2.0 m.

Maximum projection b, (Distance of suspension from protective wall, see image above)					
	perm. area load p				
Beam spacing L	<b>2</b> .0 kN/m <sup>2</sup>	3.0 kN/m²	4.0 kN/m²		
2.07 m	3.4 m	2.9 m	2.5 m		
2.57 m	3.0 m	2.6 m	2.2 m		

## THE DESIGN RESISTANCES OF THE FLEXBEAM AND THE SUSPENSION ARE:



#### Suspension

- 1. 1 x bolt dia 20 8.8 with securing pin
- 2. 2x special bolt M12-8.8 with nut or 2x hinged pin, pin dia 12 – 8.8

Design value of the resistance to tension forces of suspension  $Z_{\rm Rd}=89.2\,kN$  (59.5 kN)



Cross-section

## EXAMPLES OF POSSIBLE COMBINATIONS OF TRANSFERABLE BENDING MOMENT AND SHEAR



Intermediate values may be interpolated.

## BOLT CONNECTION BETWEEN FLEXBEAM AND STANDARD CONNECTOR (E.G. FOR THE PROTECTIVE WALL)

1 FlexBeam 2 FlexBeam standard connector 3 Bolt dia 20–8.8 with securing pin

$$\begin{split} N_{\text{Rd}} &= 68.2\,\text{kN}\;(45.5\,\text{kN})\\ \text{Value in brackets is the safe working load}\;(\gamma_{\text{F}} = 1.5) \end{split}$$



## ALUMINIUM SECTION BEAM WITH SQUARED TIMBER

The aluminium section beam with wood is a lightweight aluminium beam with low overall height for birdcage scaffolding, walkways and bridging.

#### Specifications:

Double-web beam of aluminium, 160 mm high. 1 flange 115 mm wide, with T-groove for connections with grooved bolts.

1 flange 100 mm wide, with replaceable squared timber insert, for nailed or bolted connections.

	Permissible load of aluminium section beam with squared timber							
	Span I [m]	3.0	4.0	5.0	6.0	7.0	8.0	
<u>9</u>	Uniformly distributed line load q [kN/m]	12.0	6.7	4.3	3.0	2.2	1.7	
	Deflection [cm]	2.5	4.4	6.8	9.8	13.4	17.5	
<u>à</u>	Concentrated load P in bay centre [kN]	17.9	13.4	10.7	9.0	7.7	6.7	
	Deflection [cm]	2.0	2.5	5.5	7.9	10.7	14.0	





Note: The permissible loads were calculated considerring a safety factor of  $\gamma_r=1.5$ , deflections were calculated considering  $\gamma_r=1.0.$ 

## ALLROUND FW SYSTEM

### **COMPONENTS**





## FW chord

Weight 10.5 kg - 17.4 kg Ref. No. 2646.157, 2646.207, 2646.257



FW post, extended Weight 19.0 kg Ref. No. 2646.250





FW end fitting with turnbuckle Weight 3.8 kg Ref. No. 2646.202

FW end fitting Weight 1.0 kg Ref. No. 2646.203

FW diagonal rod Weight 1.4 kg-3.3 kg Ref. No. 2646.xxx



Bolt, dia 20 mm Weight 1.6 kg, 10 pcs. Ref. No. 2646.220

Securing pin dia 4 mm Weight 1.5 kg, 50 pcs. Ref. No. 5905.001



FW post, one-sided Weight 6.4 kg – 13.8 kg Ref. No. 2646.105, 2646.155, 2646.205

FW post, one-sided, extended Weight 16.6 kg Ref. No. 2646.255





Allround FW System lock nut, dia 20 mm Weight 1.5 kg, 10 pcs. Ref. No. 2646.230

Ref. No. 2646.001

FW guardrail adapter Weight 1.2 kg

## EXAMPLE OF APPLICATION OF ALLROUND FW SYSTEM

Thanks to the modular design of the FW System and its integration into Allround Scaffolding, the scaffolding structures can be optimally adapted to building conditions. For example, vertical attachment to the open ends of the FW post is possible using Allround standards and FW post. In the transverse direction, the FW System is braced using standard Allround components. That permits the construction of bridging and support beams. With a few expansion parts, the FW System can also be used as a supporting structure for roofs.

For further information please refer to the Instructions for Assembly and Use.







Post/chord combination possibilities					
Svetom boight H of poet			Statia baight h		
System neight it of post	2.57 m	2.07 m	1.57 m		
2.0 m				1.8 m	
1.5 m				1.3 m	
1.0 m	From a structural viewpoint has no point			0.8 m	

Design resistances to tension of diagonal brace $\mathbf{Z}_{\text{Rd}}$ [kN]					
Bay length L [m]					
1.09 m	1.57 m	2.07 m	2.57 m		
123.4 (bolt connection)					
Derived design shear resistance V <sub>Z,Rd</sub> [kN]					
105.6	93.0	81.0	70.8		
94.6	78.7	65.6	55.7		
73.0 56.0 44.5 36.7					
	n resistances to 1.09 m Deriv 105.6 94.6 73.0	n resistances to tension of diago Bay leng 1.09 m 1.57 m 123.4 (bolt Derived design shear 105.6 93.0 94.6 78.7 73.0 56.0	n resistances to tension of diagonal brace Z <sub>Rd</sub> [k] Bay length L [m] 1.09 m 1.57 m 2.07 m 123.4 (bolt connection) Derived design shear resistance V <sub>Z,Rd</sub> 105.6 93.0 81.0 94.6 78.7 65.6 73.0 56.0 44.5		

Design resistances of top and bottom chords $F_{Rd}$ [kN]					
	Bay length L				
	1.09 m	1.57 m	2.07 m	2.57 m	
Compression		-123.4		-95.5	
Tension		123.4 (bolt	connection)		

### STATIC SYSTEM – PURE FRAMEWORK



Design resistances of post F <sub>Rd</sub> [kN] in the plane of the framework								
	Design resistances of end support standard (E), regular standard (R) and intermediate support standard (I)							
		End support standard E	Regular s	tandard R	Intermediate su	pport standard I		
Quatam bainht II			Bay length L <sub>1</sub>					
of nosts	Bay length $L_2$		$L_1 = 1.09 m$	$L_1 = 1.57  m$	$L_1 = 2.07 \text{ m}$	$L_1 = 2.57  m$		
01 0000		E	R/I	R/I	R/I	R/I		
2.0 m	$L_2 = 1.09 \text{ m}$	35.0	46.0/61.7	40.0/44.0	34.0/34.0	29.5/27.0		
	$L_2 = 1.57 \text{ m}$	45.5		59.0/61.7	43.5/43.0	35.0/33.0		
	$L_2 = 2.07 \text{ m}$	58.0			61.7/61.7	43.0/44.0		
	$L_2 = 2.57 \text{ m}$	42.0				56.0/61.7		
1.5 m	$L_2 = 1.09 \text{ m}$	28.0	31.0/99.0	30.0/58.0	29.0/39.0	28.0/28.0		
	$L_2 = 1.57 \text{ m}$	39.0		43.0/99.0	41.0/57.0	39.0/39.0		
	$L_2 = 2.07 \text{ m}$	58.0			70.0/99.0	58.0/58.0		
	$L_2 = 2.57 \text{ m}$	99.0				99.0/99.0		
1.0 m	$L_2 = 1.09 \text{ m}$	48.0	55.0/144.2	48.0/48.0	38.0/26.0	23.0/18.0		
	$L_2 = 1.57 \text{ m}$	144.2		144.2/144.2	46.0/46.0	25.0/25.0		
	$L_2 = 2.07 \text{ m}$	46.0			53.0/144.2	27.0/46.0		

Design resistances of posts perpendicular to the plane of the framework $F_{\mbox{\tiny Rd}}$ [kN]		Design res	sistances of posts perpendicu	lar to the plane of the framework $F_{Rd}$ [kN]
H = 2.00 m	51.8		h = 1.80 m	61.6
H = 1.50 m	81.3		h = 1.30 m	98.1
H = 1.00 m	126.6		h = 0.80 m	144.2

## FREE-STANDING TOWERS

#### Version with bracket (B) Side view



Version with Allround (A) Side view



Version with bracket (B) Version with Allround (A) Front view



Free-standing towers Platform height: 2.25 m	Bay length L: 2.57 m				
		Outdoors		Indoors	
Bay width a [m]	Cantilever arm length c [m]	Ballast total [kg]	Max. load to a standard [kN]	Ballast total [kg]	Max. load to a standard [kN]
1.57	0.39 (B)	370	6.4	-	5.2
	0.73 (B)	490	8.2	45	6.9
2.07	0.39 (B)	100	6.4	-	6.0
	0.73 (B)	190	8.1	-	7.5
2.57	0.00	-	5.5	-	5.4
	0.39 (B)	-	6.9	-	6.9
	0.73 (B)	-	8.3	-	8.3
Platform height: 4.25 m	Bay length L: 2.57 m				
1.57	0.39 (B)	1400	10.4	-	6.2
	0.73 (B)	1515	12.2	-	7.8
	0.73 (A)	1595	12.8	95	8.4
2.07	0.39 (B)	745	9.2	-	6.9
	0.73 (B)	835	10.9	-	8.4
	0.73 (A)	895	11.4	-	8.8
	1.09 (A)	1050	13.5	115	10.8
	1.57 (A)	1340	16.8	780	15.1
2.57	0.00	275	7.4	-	6.4
	0.39 (B)	330	8.9	-	7.7
	0.73 (B)	405	10.4	-	9.2
	0.73 (A)	450	10.9	-	9.5
	1.09 (A)	580	12.8	-	11.1
	1.57 (A)	810	15.8	360	14.5
	2.07 (A)	1330	19.8	1090	19.5
	2.57 (A)	2230	25.1	2025	24.6

Basis for dimensioning of free-standing towers:	0.000 hN//2
The assembly variants for outdoor use were calculated assuming the dynamic pressure as per EN 12810-1, Fig. 3,	q = 0.623 kN/m <sup>2</sup>
reduced by a factor of 0.7 to take into account a time from the erection to the dismantling of $\leq 2$ years:	E
$q_{(H=0)} = 0.800 \text{ kN/m}^2 \text{ x } 0.7 = 0.560 \text{ kN/m}^2$	7.25
$q_{(H=7.25)} = 0.891 \text{ kN/m}^2 \times 0.7 = 0.623 \text{ kN/m}^2$	
In Germany this covers: Wind zone 3, inland, time from the erection to the dismantling $\leq$ 2 years	⊈ q = 0.560 kN/m²

Free-standing towers					
Platform height: 6.25 m	Bay length L: 2.57 m				
		Outdoors		Indoors	
Bay width a [m]	Cantilever arm length c [m]	Ballast total [kg]	Max. load to a standard [kN]	Ballast total [kg]	Max. load to a standard [kN]
1.57	0.39 (B)	2980	17.7	-	7.2
	0.73 (B)	3095	18.6	-	8.8
	0.73 (A)	3175	19.2	70	9.4
2.07	0.39 (B)	1880	13.8	-	7.9
	0.73 (B)	1970	15.3	-	9.4
	0.73 (A)	2030	15.8	-	9.8
	1.09 (A)	2190	17.9	40	11.7
	1.57 (A)	2480	21.1	715	16.1
2.57	0.00	1150	11.1	-	7.5
	0.39 (B)	1200	12.5	-	8.8
	0.73 (B)	1270	14.1	-	10.3
	0.73 (A)	1320	14.6	-	10.7
	1.09 (A)	1445	16.4	-	12.3
	1.57 (A)	1680	19.4	265	15.4
	2.07 (A)	1985	22.9	1000	20.1
	2.57 (A)	2395	27.0	1950	25.7

## BRIDGING

#### Front and side view

Detail of base point

Section A-A (bracing with tube and coupler)



 $q = 0.606 \text{ kN/m}^2$ 

q = 0.560 kN/m<sup>2</sup>

5.25 m\_

#### Bridging

Platform height: 4.25 m					
		Outdoors		Indoors	
Bay width a [m]	Bay length L [m]	Ballast total [kg]	Max. load to a standard [kN]	Ballast total [kg]	Max. load to a standard [kN]
1.57	4.14 (2 x 2.07)	820	10.4	-	10.8
	5.14 (2 x 2.57)	930	11.9	-	12.6
	6.14 (2 x 3.07)	1040	13.1	-	14.2
	7.71 (3 x 2.57)	1200	13.8	-	11.3
2.07	4.14 (2 x 2.07)	920	10.9	-	11.8
	5.14 (2 x 2.57)	1030	12.3	-	13.6
	6.14 (2 x 3.07)	1140	13.4	-	15.2
	7.71 (3 x 2.57)	1290	13.9	-	11.9
2.57	4.14 (2 x 2.07)	1020	11.8	-	12.8
	5.14 (2 x 2.57)	1140	13.2	-	14.6
	6.14 (2 x 3.07)	1240	14.3	-	16.3
	7.71 (3 x 2.57)	1400	14.7	-	12.5

#### Basis for dimensioning of bridging:

The assembly variants for outdoor use were calculated assuming the dynamic pressure as per EN 12810-1, Fig. 3, reduced by a factor of 0.7 to take into account a time from the erection to the dismantling of  $\leq$  2 years:

$$q_{(H=0)} = 0.800 \text{ kN/m}^2 \text{ x } 0.7 = 0.560 \text{ kN/m}^2$$

 $q_{(H=5.25)} = 0.866 \text{ kN/m}^2 \text{ x } 0.7 = 0.606 \text{ kN/m}^2$ In Germany this covers: Wind zone 3, inland, time from the erection to the dismantling  $\leq 2$  years

## **HEAVY-DUTY TOWER**

Permissible vertical load F[kN] per Allround heavy-duty tower 1.09 x 1.09 m, laterally held at the top						
Tower height [m]	Dynamic wind pressure q <sub>w</sub> [kN/m <sup>2</sup> ]					
	no wind	0.5	0.8	1.2		
2	602.0	593.6	588.8	582.4		
4	564.4	548.8	544.0	536.0		
6	564.4	555.6	540.4	481.2		
8	554.8	518.4	452.0	363.6		
10	535.2	436.8	352.8	240.8		
12	518.0	398.8	290.0	145.6		
16	504.0	298.0	144.8	-		
20	492.4	201.6	-	-		

1.09 m #601 1.24 m

Double wedge head coupler spacing 50 cm or 100 cm Spindle extension of head jack and base plate  $a \le 0.25$  m



F/4

F/4

## SUPPORT SCAFFOLDING

## SUPPORTING A FREE-STANDING FACADE WITH ALLROUND SCAFFOLDING

A free-standing wall or facade can be supported very effectively with Allround Scaffolding, e.g. when renovating historic buildings. The support scaffolding must sustain the wind loads on the facade. This requires a static calculation for the specific project. The scaffolding must be connected to the facade, as shown below left.



Connection of the scaffolding to the free-standing facade





Example of support scaffolding for a free-standing facade

To ensure the positional stability of support scaffolding, it has to be ballasted in accordance with the static calculation.

The support scaffolding must always use Allround standards with bolt-in spigots or bolted LW standards!

#### The required weight of the ballast depends on:

- the height of the wall
- the space available on the ground for widening the scaffolding
- the wind load

#### The structural design of the level(s) for laying the ballast depends on:

> the load-bearing capacity of the decks, deck ledger or lattice beams on which the ballast is placed

#### Notes on ballasting with positioned ballast weights:

- > Do not place the ballast at the level of the base collars, as this does not allow any tension forces to be transferred (see Fig. below)
- > Ballast is usually made of solid materials such as concrete or steel
- Static calculation required

### **WRONG**



RIGHT



### BALLASTING WITH CONCRETE FOUNDATION:

Sometimes it is not possible to make support scaffolding sufficiently stable using placed ballast weights. The load capacity of the ground area, and the permissible load of the Allround standards and of the scaffolding deck area for positioning of the ballast weights would be exceeded. This can happen in particular when only a limited setup area is available for the scaffolding or if the scaffolding width has to be limited for other reasons. In these cases the scaffolding standards can be embedded in a concrete foundation.

Support scaffolding with concrete foundation must of course also be verified by static calculation.



Example: Casting the scaffolding standards in a concrete foundation

# LAYHER ALLROUND SCAFFOLDING® IN USE more possibilities



Layher Allround Scaffolding ensures shorter assembly times, optimises costs, increases safety when enclosing churches, monuments and all kinds of towers – scaffolding on and in boilers, storage tanks and pipelines, scaffolding over workplaces and traffic routes, around machines and / or under bridges – construction scaffolding or rolling tunnel structures: There is no job that can't be done more quickly, economically and safely with the Layher Allround system.

The building industry puts high demands for load-bearing capacity and design variation in scaffolding. This is where Allround Scaffolding is setting the standards: one system, as bricklayers' scaffolding, work scaffolding or protective scaffold with bay widths of 0.73 m, 1.09 m or 1.40 m, with selectable lift heights and live loads up to load class 6, depending on the bay width. Or assembled as scaffolding for formwork or support: with Allround Scaffolding you're prepared for anything.



Mobile tunnel scaffolding

## BIRDCAGE SCAFFOLDING REPAIR – RENOVATION



Building repair is an ongoing task. With Allround Scaffolding you can get on with any job. Concrete repair work on major structures, and the renovation of historic structures of all types, such as half-timbered houses, churches, castles and museums, including the restoration of artistically and historically valuable ceilings, or internal or external scaffolding for asbestos clearance.

## ENGINEERING SCAFFOLDING

SCAFFOLDING FOR DEMANDING BUILDING SHAPES, SUCH AS STEEPLES AND DOMES.



For scaffolding around and inside churches in particular, Allround Scaffolding offers impressive flexibility plus simpler and safer handling. With its particular benefits, such as rapid assembly without bolts, positive and non-positive connections, dimensional accuracy and stiffness, you can rapidly create safer workplaces for roofers, masons, carpenters, plasterers, plumbers, painters and glaziers – both indoors and outdoors – even at extreme heights.

## **INDUSTRIAL SCAFFOLDING** SAFE WORKPLACES AND ASSEMBLY PLACES



Industrial scaffolding is used for a wide range of applications, for example tall machinery and plant has to be serviced and repaired, equipment and systems have to be assembled, electrical units have to be replaced.

Using the Allround System, safer places for work and assembly can be created in a very short time in any industrial facility and in any company, whatever its size and whatever the industry. Here today, there tomorrow – wherever it's used, it permits faster work thanks to a secure footing at height.

## AS A BASIC SYSTEM FOR VERSATILE USE STAIRTOWERS – ROLLING TOWERS – CLADDING WITH PROTECT



The great variability of Allround Scaffolding means that a wide range of applications can be handled using a small number of additional parts. By using stair stringers and appropriate guardrail, stairtowers ranging from construction stairtowers to stairs in areas open to the public can be built. Rolling towers with a range of ground plans and heights are possible. Together with the Protect system, waterproof enclosures covering entire facades, e.g. for asbestos clearance, can be realised.



## SHIPYARDS AND THE OFFSHORE SECTOR SHORT INSTALLATION TIMES – FOR VERY FAST REPAIRS



In addition to Layher Allround Scaffolding, we offer Layher application technology, including technical consultation with qualified, trained contact partners. At your headquarters, at your building site, in your nearest Layher branch, or in the central technical office. Or highly experienced supervisors who help you to fully exploit the profitable possibilities of the Allround system.

One particular focus of the Allround Scaffolding is the construction of racks in shipyards and in the offshore sector. Enclosing the difficult shapes on and inside a ship, above and below deck, on and underneath offshore platforms, are no problem for the Allround Scaffolding, any more than the fast assembly and dismantling times that are required. For maintenance on drilling rigs, offshore or in the repair yard, the Allround Scaffolding is nowadays indispensable due to its versatility and adaptability.



## **OPTIMUM SCAFFOLDING FOR AIRCRAFT** SAFETY – RELIABILITY – ECONOMY



Safety and service are vital when it comes to aircraft. This not only applies to the flight itself, but also to maintenance and therefore to the maintenance equipment. Whether for mobile maintenance units or special structures, Layher Allround Scaffolding is the right choice wherever more reliability and safer work at exactly the right height is critical.

#### Flexibility due to

- variable working heights
- selectable bay lengths and widths
- > perfect adjustment to the contours of the aircraft

#### Reliability and improved safety thanks to

- bolt-free connection technology
- short assembly and dismantling times
- non-slip decks, comfortable stairs, strong castor wheels and other components from a well-thought-out and mature system

Layher Allround Scaffolding is outstandingly suitable for aircraft maintenance.



## STANDS. PODIUMS. EVENT STRUCTURES FOR **INDOORS AND OUTDOORS**

FOR EVERY OCCASION IN THE EVENTS SECTOR



Using the Layher Allround system, you can safely, inexpensively and quickly assemble mobile stands, podiums and event structures of widely varying types for indoors and outdoors, for any occasion, in variable sizes.

Matching roof structures are available as the Keder Roof XL, cassette roof or Allround FW System roof - in mono-pitch or double-pitch design, made from standard Layher material.



Event stage





Proximity to the customer is a central factor behind Layher's success – geographically speaking too. Wherever our customers need us, we will be there – with our advice, assistance and solutions.

Layher is your dependable partner with more than 70 years of experience. "Made by Layher" always means "Made in Germany" too – and that goes for the entire product range. Superb quality – and all from one source.





## Layher. 🕅

More Possibilities. The Scaffolding System.

Wilhelm Layher GmbH & Co KG Scaffolding Grandstands Ladders

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